GENERAL NOTES

ELEVATION DATUM

SEE ARCHITECTURAL DRAWINGS OR SITE PLAN FOR FINISH FLOOR ELEVATIONS.

DESIGN SPECIFICATIONS

2006 INTERNATIONAL BUILDING CODE WITH 2008 WISCONSIN AMMENDMENTS.

EARTHWORK

EARTHWORK OPERATIONS SHALL BE PERFORMED UNDER THE DIRECTION OF A PROFESSIONAL TESTING AGENCY TO ASSURE COMPLIANCE WITH THE RECOMMENDATIONS OF THE SOILS REPORT BY CGC, INC. DATED JULY 23, 2003 (PROJECT NO. C03164).

FOOTINGS

1. ALL FOOTINGS SHALL BEAR ON UNDISTURBED SOIL OR ENGINEERED FILL.
2. HORIZONTAL REINFORCING IN FOOTINGS SHALL BE CONTINUOUS AT CORNERS AND INTERSECTIONS. CORNER BARS SHALL BE PROVIDED TO MATCH HORIZONTAL STEEL. REINFORCING STEEL SHALL BE LAPPED AS FOLLOWS WHERE SPLICES ARE REQUIRED:

BAR SIZE	LAP DIMENSION
#4	1'-6"
#5	1'-9"
#6	2'-0"

CONCRETE

CONCRETE WORK SHALL CONFORM TO ALL REQUIREMENTS OF THE CURRENT ACI 301, SPECIFICATIONS FOR STRUCTURAL CONCRETE FOR BUILDINGS, ACI 318 BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE, ACI 305 SPECIFICATIONS FOR HOT WEATHER CONCRETE, AND ACI 306 SPECIFICATIONS FOR COLD WEATHER CONCRETE, WITH THE FOLLOWING ADDITIONAL REQUIREMENTS:

1. CONCRETE SHALL DEVELOP THE FOLLOWING 28-DAY MINIMUM COMPRESSIVE STRENGTH:
   - FOUNDATIONS – 3,000 PSI
   - FOUNDATION WALLS – 4,000 PSI
   - FLOOR SLAB – 4,000 PSI
   - WALL PANELS – 3,500 PSI U.N.O.
2. AIR CONTENT FOR WALL PANELS SHALL BE NATURAL AMOUNTS NOT TO EXCEED 4%.
3. CHLORIDE-BASED ADMIXTURES ARE PROHIBITED IN ALL REINFORCED CONCRETE.
4. REINFORCING STEEL SHALL CONFORM TO ASTM A615, A616, OR A617, GRADE 60.
5. CONCRETE EXPANSION ANCHORS, SIZE AS PER PLAN, SHALL DEVELOP THE FOLLOWING MINIMUM WORKING LOAD CAPACITIES IN 4000 PSI CONCRETE:

DIA.	TENSION	SHEAR
½"	1,450#	1,787#
⅝"	2,091#	2,973#
¾"	2,670#	3,765#

FASTENERS THAT MEET THESE REQUIREMENTS ARE "POWER-STUD" BY THE POWERS RAWL DIVISION OF POWER FASTENING, INC. AND "KWIK-BOLT 3" BY HILTI, INC. ANCHORS SHALL BE INSTALLED PER MANUFACTURER'S RECOMMENDATIONS WITH PARTICULAR ATTENTION TO PROPER TORQUE.
IF SPECIFIC ANCHORS ARE SHOWN ON DRAWINGS, THEY MUST BE USED UNLESS AN ALTERNATE IS APPROVED BY THE ENGINEER.

STRUCTURAL STEEL

1. FABRICATION AND ERECTION OF STRUCTURAL STEEL SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF THE AISC SPECIFICATIONS FOR STRUCTURAL STEEL BUILDINGS, THE AISC CODE OF STANDARD PRACTICE FOR STEEL BUILDINGS AND BRIDGES AND CURRENT OSHA STANDARDS.
2. WIDE FLANGE SHAPES SHALL CONFORM TO ASTM A992. STRUCTURAL TUBES SHALL CONFORM TO ASTM A500 GRADE B. ALL OTHER STRUCTURAL STEEL SHALL CONFORM TO ASTM A36.
3. BOLTS, UNLESS OTHERWISE SHOWN, SHALL CONFORM TO ASTM A325-N, SIZE AS PER PLAN.
4. ANCHOR RODS, UNLESS OTHERWISE SHOWN, SHALL CONFORM TO ASTM F1554 GRADE 36.
5. SPLICING OF STRUCTURAL STEEL IS PROHIBITED EXCEPT AS DETAILED.
6. ALL STRUCTURAL AND MISCELLANEOUS STEEL ITEMS SHALL RECEIVE ONE COAT OF "IRONCLAD RETARDO RUST INHIBITIVE PAINT 163" (BENJAMIN MOORE) OR APPROVED EQUAL UNLESS OTHERWISE INDICATED IN THE SPECIFICATIONS. ALL STEEL SURFACES EMBEDDED IN CONCRETE SHALL NOT BE PAINTED. PREPARATION OF STEEL SURFACES SHALL MEET THE REQUIREMENTS OF THE STEEL STRUCTURES PAINTING COUNCIL (SSPC). THESE INCLUDE THE REMOVAL OF GREASE AND OIL BY SOLVENT CLEANING (SSPC-SP1) AND THE REMOVAL OF MILL SCALE, RUST, WELD FLUX AND SLAG BY HAND TOOL CLEANING (SSPC-SP2). PRIMER SHALL BE APPLIED AT THE MANUFACTURER'S RECOMMENDED RATE BUT NOT LESS THAN ONE GALLON PER 400 SQ. FT. THEREBY DEPOSITING A DRY FILM THICKNESS OF NOT LESS THAN 1.5 MILS. ANY SCARRED AREAS SHALL BE TOUCHED UP WITH THE SAME PAINT AFTER ERECTION.
7. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS IN ACCORDANCE WITH THE CURRENT EDITION OF THE AWS STRUCTURAL WELDING CODE. WELDING ELECTRODES SHALL BE E70XX.
8. ALL ROOF OPENINGS SHALL BE FRAMED WITH STRUCTURAL STEEL, SIZED AS REQUIRED. THE LOCATION AND SIZE OF ROOF OPENINGS SHALL BE APPROVED BY THE STRUCTURAL ENGINEER PRIOR TO INSTALLATION.
9. TEMPORARY VERTICAL CROSS BRACING PER AISC (½" 63/64 CABLES) IS REQUIRED ALONG EVERY COLUMN LINE AT EVERY 150' MAX. IF STRUCTURAL STEEL IS ERECTED PRIOR TO PANELS. BRACING IS REQUIRED IN EACH DIRECTION AND SHALL REMAIN IN PLACE UNTIL THE STRUCTURAL STEEL IS CONNECTED TO THE WALL PANELS.

STEEL JOISTS AND JOIST GIRDERS

1. THE DESIGN, FABRICATION AND ERECTION OF STEEL JOISTS AND JOIST GIRDERS SHALL CONFORM TO THE REQUIREMENTS OF THE CURRENT EDITION OF THE STANDARD SPECIFICATIONS AND RECOMMENDED CODE OF STANDARD PRACTICE FOR OPEN WEB JOISTS AND JOIST GIRDERS ADOPTED BY THE STEEL JOIST INSTITUTE, AND CURRENT OSHA STANDARDS.
2. NO CONSTRUCTION LOADS SHALL BE PLACED ON JOISTS OR JOIST GIRDERS UNTIL BRIDGING IS INSTALLED AND BEARING CONNECTIONS HAVE BEEN BOLTED OR WELDED.
3. JOIST BRIDGING BUNDLES SHALL NOT EXCEED 1,000 LBS.

ROOF DRAINAGE

PROVISION SHALL BE MADE FOR SECONDARY ROOF DRAINAGE BY MEANS OF OVERFLOW SCUPPERS IN WALLS OR ADDITIONAL INTERIOR DRAINS. (SEE MECHANICAL DRAWINGS.) HEIGHT OF SECONDARY DRAINS ABOVE PRIMARY DRAINS SHALL BE SUCH THAT THE WEIGHT OF PONDED WATER ON THE ROOF DOES NOT EXCEED THE DESIGN LIVE LOAD.

STEEL ROOF DECK

1. THE DESIGN, FABRICATION AND ERECTION OF THE STEEL ROOF DECK SHALL BE IN ACCORDANCE WITH THE CURRENT EDITION OF THE SDI SPECIFICATIONS AND COMMENTARY FOR STEEL ROOF DECK AND THE SDI DIAPHRAGM DESIGN MANUAL.
2. MINIMUM END LAP SHALL BE 3".
3. THE STEEL ROOF DECK FUNCTIONS AS A STRUCTURAL ELEMENT IN RESISTING LATERAL LOADS AND PROVIDES OVERALL STABILITY FOR THE BUILDING. THEREFORE THE WALL PANEL ERECTION BRACES SHALL NOT BE REMOVED UNTIL ALL STEEL DECK IS COMPLETELY FASTENED IN PLACE.
4. ALL WELDING SHALL BE DONE BY QUALIFIED WELDERS IN ACCORDANCE WITH THE CURRENT EDITION OF THE AWS SPECIFICATIONS FOR WELDED SHEET STEEL AND ITS COMMENTARY. WELDING ELECTRODES SHALL BE E6022. HOBART #1139, 5/32" DIA. WELDING ELECTRODES MEET THIS REQUIREMENT.
5. ROOF DECK SHALL RECEIVE ONE COAT OF MANUFACTURER'S STANDARD PRIMER. ALL DECK WELDS SHALL BE PAINTED WITH RUST PROHIBITIVE METAL PRIMER PRIOR TO ROOFING.
6. ROOF DECK SHALL BE CONTINUOUS OVER A MINIMUM OF THREE SPANS.
7. ROOF DECK BUNDLES SHALL BE PLACED ON JOISTS WITH EXTREME CAUTION, FOLLOWING THE JOIST MANUFACTURER'S RECOMMENDATIONS FOR PROPER PLACEMENT.
8. DECKING OR DECK ACCESSORY BUNDLES SHALL NOT EXCEED 4,000 LBS.

INDEX OF SHEETS

GENERAL STRUCTURAL DATA	S100
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JOIST AND JOIST GIRDER SCHEDULES	S303
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KEY PLAN	S400
WALL PANEL LAYOUT	S401
RELOCATED WALL PANEL LAYOUT	S402
WALL PANEL REINFORCING	S500
WALL PANEL INSERTS	S600
STANDARD FABRICATION SHEET	S700

DESIGN DATA

ROOF LOAD

LIVE LOAD	30
1 PLY MEMB. (BALLASTED), INSULATION & DECK	16
MECHANICAL ALLOWANCE	5
BAR JOISTS	3
TOTAL TO JOISTS	54 LBS./FT²
BEAMS OR JOIST GIRDERS	2
TOTAL TO BEAMS OR JOIST GIRDERS	56 LBS./FT²

BUILDING CATEGORY

II

ROOF SNOW LOAD

(DRIFTING SNOW IN ADDITION TO UNIFORM LOAD WHERE APPLICABLE)

P_g = 35 LBS./FT²
P_f = 27 LBS./FT²
C_e = 1.1
I_s = 1.0
C_t = 1.0

BASIC DESIGN WIND LOAD

V = 90 M.P.H. (3-SECOND GUST)
I_w = 1.0
EXPOSURE C
INTERNAL PRESSURE COEFFICIENT = ± 0.18
NON-STRUCTURAL COMPONENTS AND CLADDING SHALL BE DESIGNED FOR:
20.2 LBS./FT² PRESSURE
27.0 LBS./FT² SUCTION

ALLOWABLE SOIL BEARING

3,500 LBS./FT²

EARTHQUAKE DESIGN DATA

S_S= 0.105
S_1= 0.044
SITE CLASS D (UNKNOWN)
S_{DS}= 0.111
S_{D1}= 0.070
SEISMIC DESIGN CATEGORY B
BASIC SEISMIC-FORCE-RESISTING SYSTEM = ORDINARY PRECAST CONCRETE SHEAR WALLS
I_E = 1.0
R = 3
V = C_SW = 0.037W
EQUIVALENT LATERAL FORCE PROCEDURE

CLEAR HEIGHT

FINISH FLOOR TO BAR JOIST (WAREHOUSE)	24'-6 1/2" (MIN.)
FINISH FLOOR TO JOIST GIRDER (WAREHOUSE)	23'-6" (MIN.)
FINISH FLOOR TO BAR JOIST (OFFICE)	12'-4 1/2" (MIN.)
FINISH FLOOR TO BEAM (OFFICE)	12'-8" (MIN.)

BUILDING FLOOR AREA

SLAB ON GRADE (WAREHOUSE ADDITION)	16,956 SQ. FT.
SLAB ON GRADE (OFFICE ADDITION)	1,288 SQ. FT.
TOTAL	18,244 SQ. FT.

PROJECT RECORD
09.14.09

CON/STEEL ALLIANCE

REVISIONS

NO.	DATE	DESCRIPTION	BY
3	2-10-09	RELEASED FOR CONSTRUCTION	STR
2	10-7-08	RELEASED FOR PERMIT	STR
1	8-14-08	RELEASED FOR REVIEW	STR

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SHEET TITLE	GENERAL STRUCTURAL DATA		
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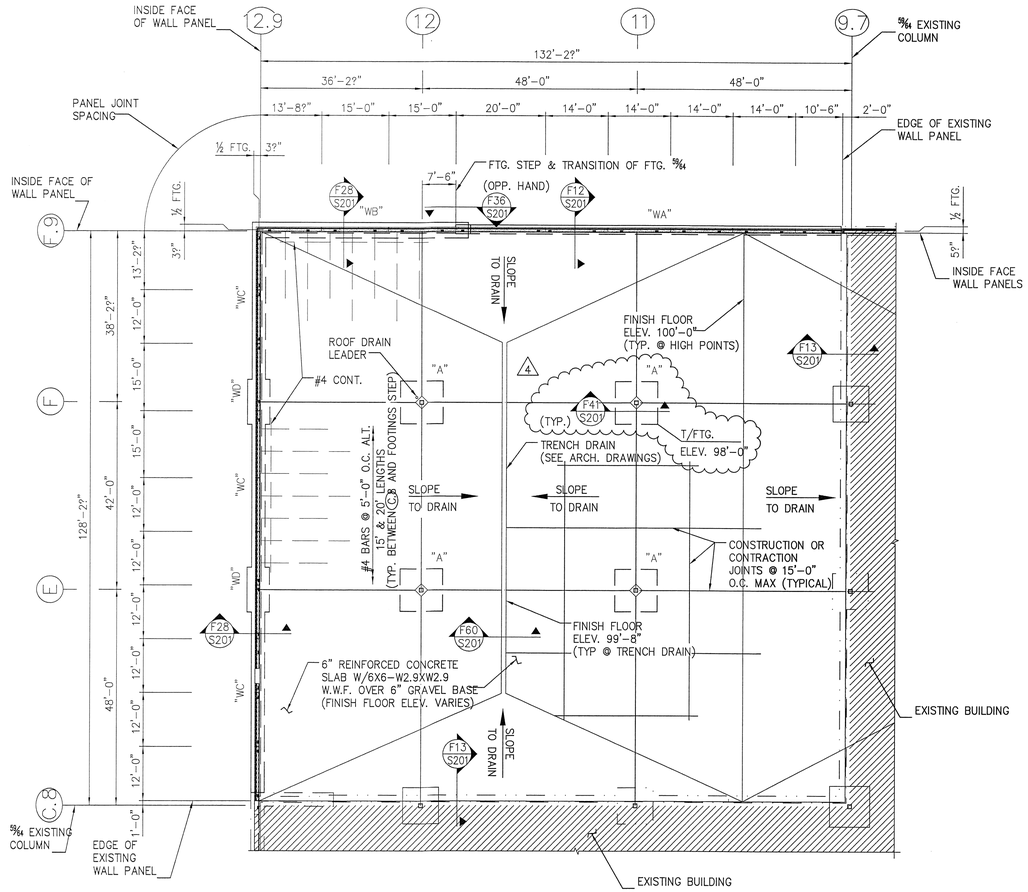
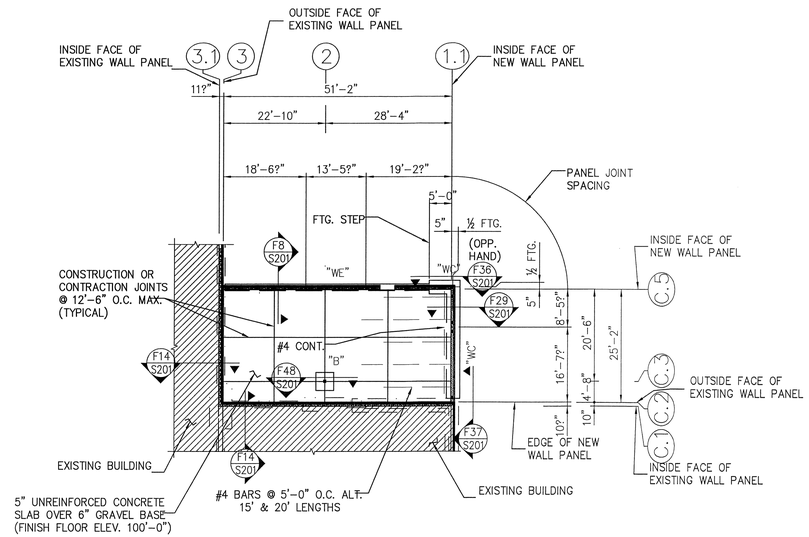
DRAWN	JMH		
CHECKED	STR	SHEET NO.	S100
DATE	AUG. 08		



NOTE:

EPOXY REINFORCING STEEL FROM NEW FOUNDATIONS AND FOUNDATION WALLS INTO EXISTING FOUNDATIONS AND FOUNDATION WALLS

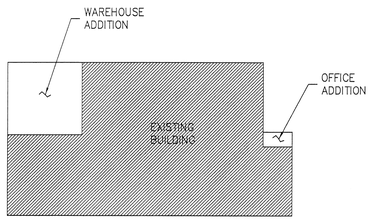


NOTE:

EPOXY REINFORCING STEEL FROM NEW FOUNDATIONS AND FOUNDATION WALLS INTO EXISTING FOUNDATIONS AND FOUNDATION WALLS

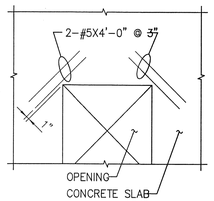
NORTH

FOOTING SCHEDULE				
MARK	WIDTH (W)	LENGTH (L)	DEPTH (D)	REINFORCING
"A"	9'-6" SQ.	—	2'-0"	12-#6X7'-6" EACH WAY
"B"	4'-0" SQ.	—	2'-0"	7-#5X3'-6" EACH WAY
"WA"	2'-0"	CONT.	3'-6"	3-#5 CONT. TOP & BOTTOM
"WB"	3'-6"	CONT.	3'-6"	6-#5 CONT. TOP & BOTTOM
"WC"	3'-0"	CONT.	3'-6"	5-#5 CONT. TOP & BOTTOM
"WD"	5'-0"	SEE PLAN	3'-6"	5-#5 CONT. & 3-#5X9'-6" ADD. TOP & BOTTOM
"WE"	1'-6"	CONT.	3'-6"	3-#5 CONT. TOP & BOTTOM



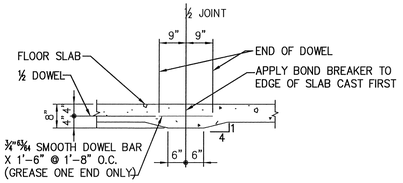
KEY PLAN

NORTH



CRACK CONTROL REINFORCING

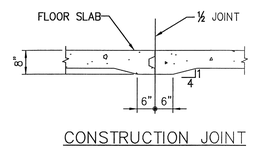
NOTE: TYPICAL AT ALL REENTRANT CORNERS FOR SLAB-ON-GRADE. REINFORCING TO BE CENTERED IN SLAB THICKNESS.



CONSTRUCTION JOINT

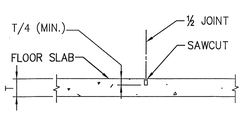
PROCEDURE FOR PLACING DOWELS IN CONSTRUCTION JOINT

1. GREASE HALF OF DOWEL PROTRUDING THROUGH THE EDGE OF FORMS INTO THE FIRST SLAB POUR.
2. AFTER CONCRETE HAS REACHED INITIAL SET, TWIST DOWELS IN SLAB SEVERAL TURNS TO ENSURE EASE OF REMOVAL. REMOVE DOWELS AFTER 3 TO 4 HOURS.
3. STRIP FORMS AND IMMEDIATELY REINSERT DOWELS INTO SLAB.



CONSTRUCTION JOINT

ALTERNATE DETAIL AT CONTRACTOR OPTION



CONTRACTION JOINT

REVISIONS			
NO.	DATE	DESCRIPTION	BY
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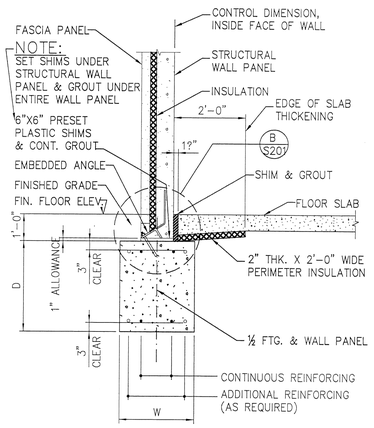
PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

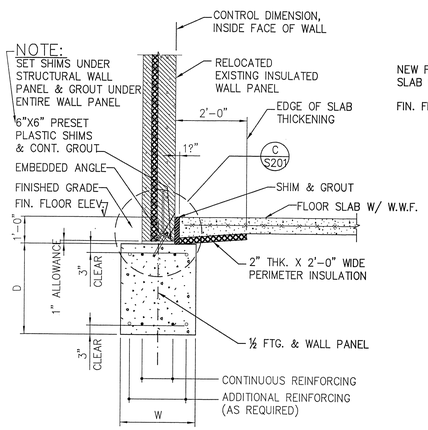
SHEET TITLE: FOUNDATION PLAN

DESIGNED JML	JOB NO. 103220
DRAWN JMH	
CHECKED STR	SHEET NO. S200
DATE JUL 08	



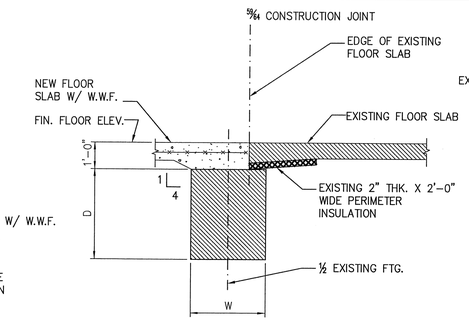
WALL PANEL FOOTING SECTION AT OFFICE — F8 / S200

NOTE:
SEE FOUNDATION PLAN FOR W, D AND REINFORCING STEEL.

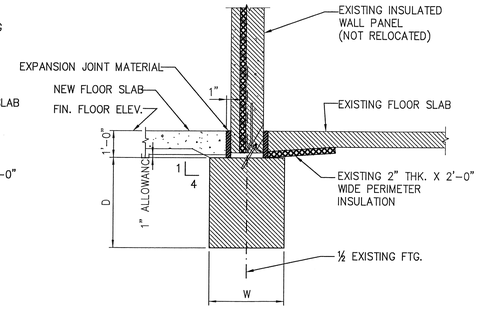


WALL PANEL FOOTING SECTION AT WAREHOUSE — F12 / S200

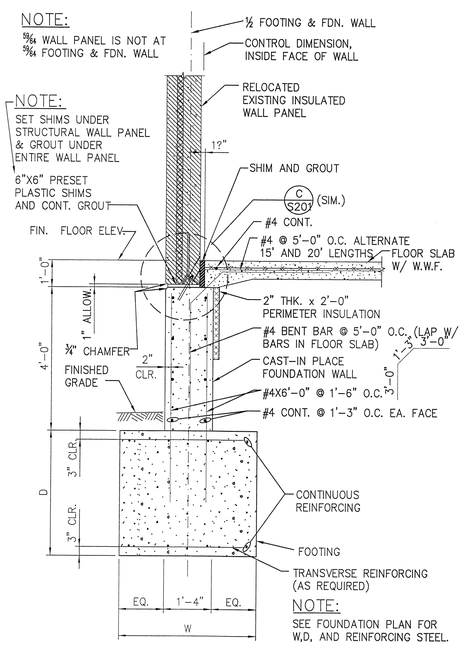
NOTE:
SEE FOUNDATION PLAN FOR W, D AND REINFORCING STEEL.



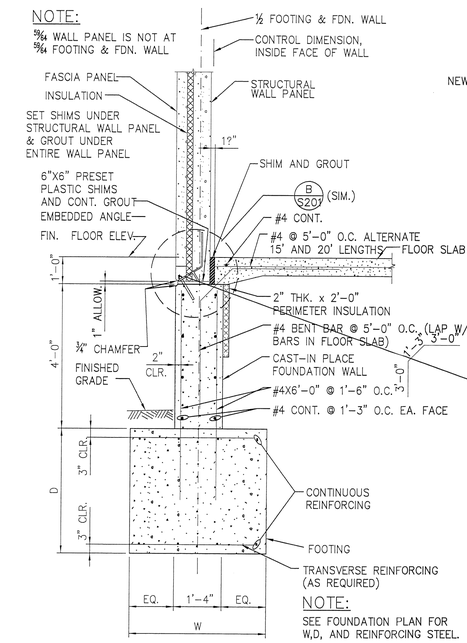
WALL FOOTING SECTION AT WAREHOUSE — F13 / S200



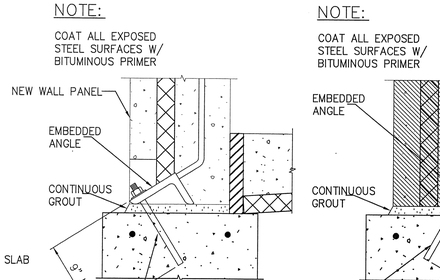
WALL FOOTING SECTION AT OFFICE — F14 / S200



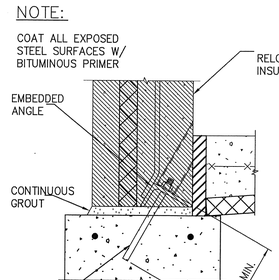
WALL PANEL FOOTING SECTION AT WAREHOUSE — F28 / S200



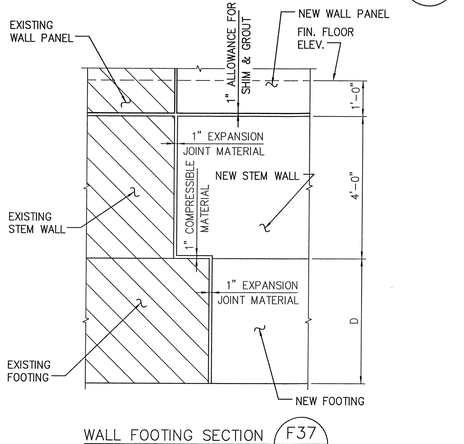
WALL PANEL FOOTING SECTION AT OFFICE — F29 / S200



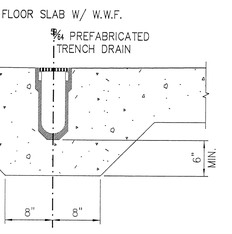
DETAIL — B / S201



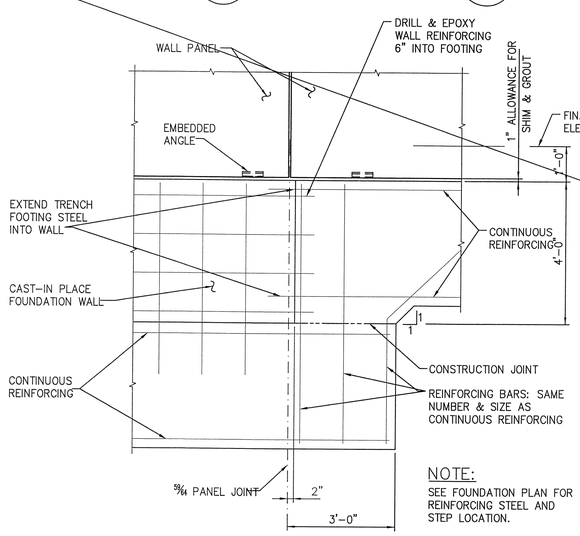
DETAIL — C / S201



WALL FOOTING SECTION — F37 / S200

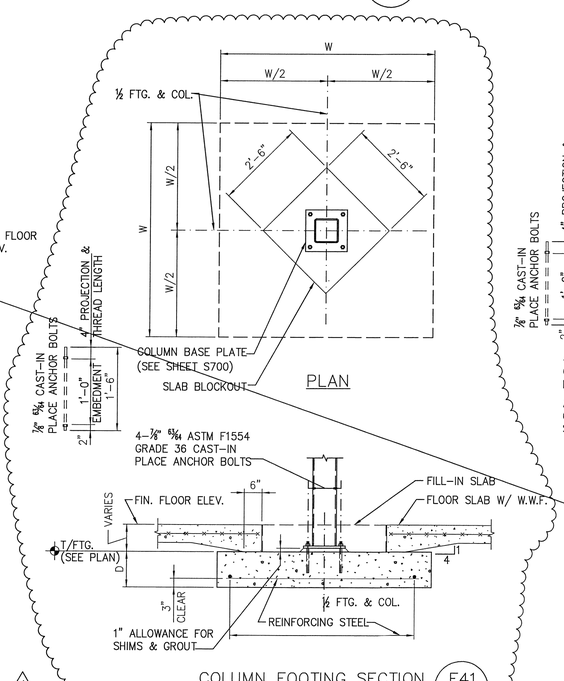


TRENCH DRAIN SECTION — F60 / S200



NOTE:
SEE FOUNDATION PLAN FOR REINFORCING STEEL AND STEP LOCATION.

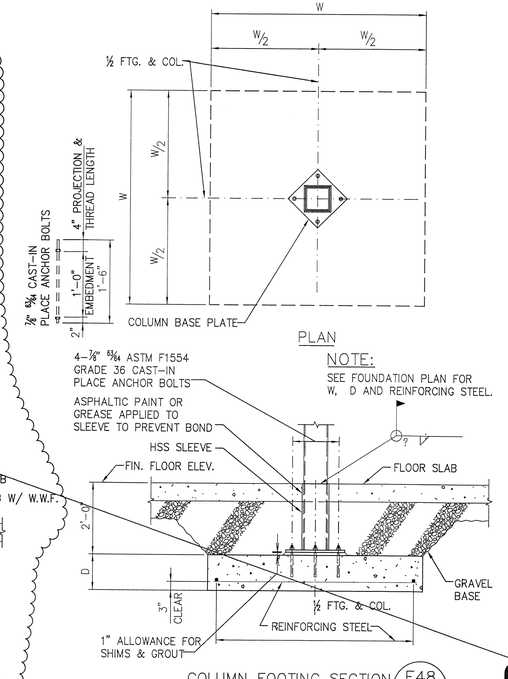
FOOTING STEP SECTION — F36 / S200



4

COLUMN FOOTING SECTION AT WAREHOUSE — F41 / S200

NOTE:
SEE FOUNDATION PLAN FOR W, D AND REINFORCING STEEL.



NOTE:
SEE FOUNDATION PLAN FOR W, D AND REINFORCING STEEL.

COLUMN FOOTING SECTION AT OFFICE — F48 / S200

PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

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4	2-10-09	RELEASED FOR CONSTRUCTION	STR
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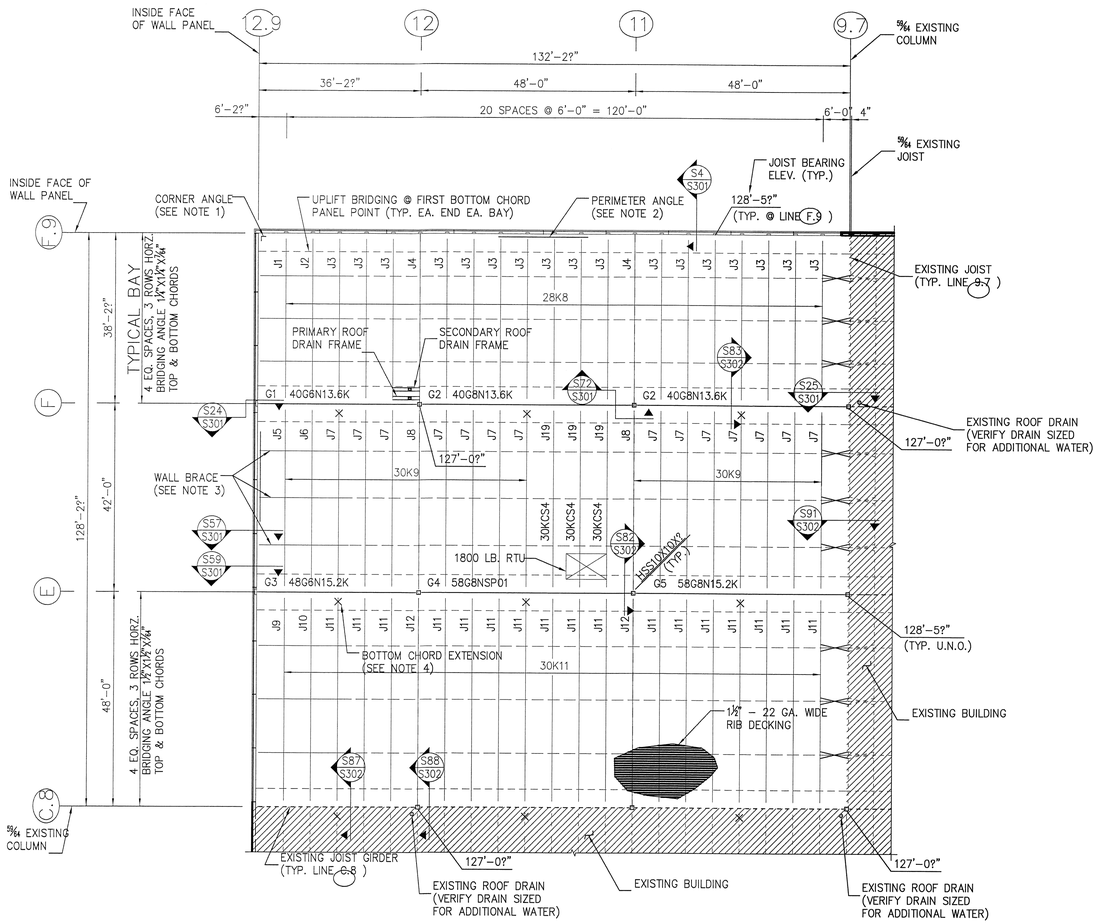
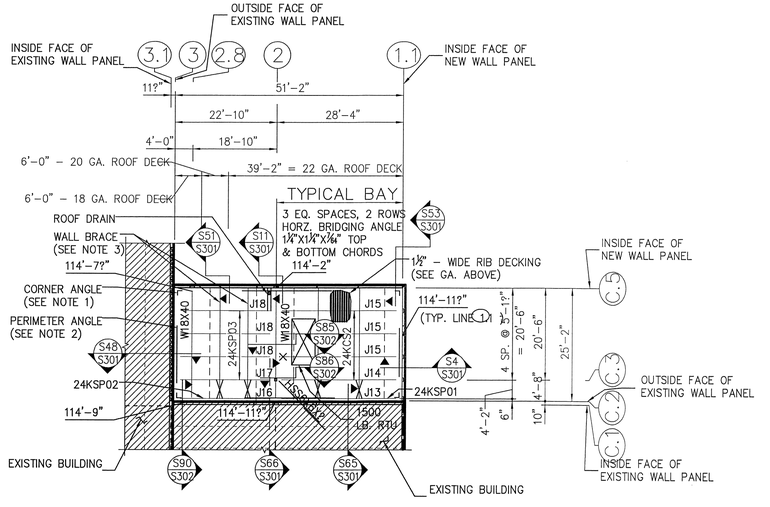
SHEET TITLE: FOUNDATION DETAILS

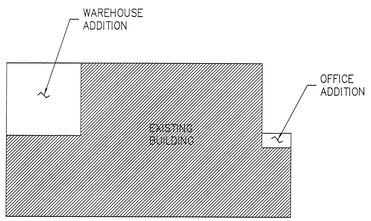
DESIGNED	JML
DRAWN	JMH
CHECKED	STR
DATE	JUL. 08
JOB NO.	103220
SHEET NO.	S201



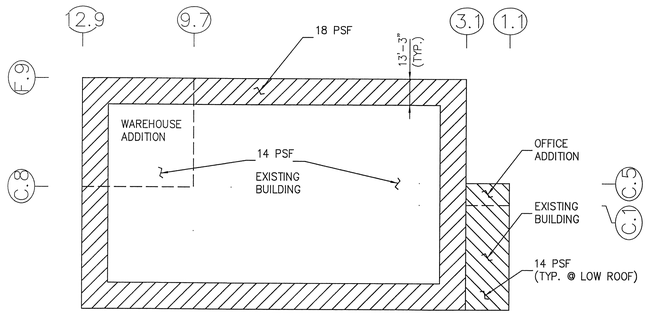


NOTES:

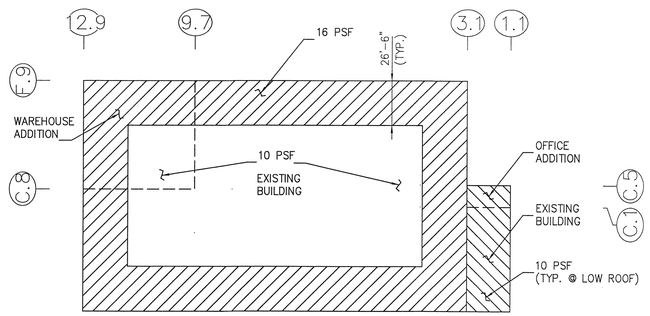
1. INSTALL CORNER ANGLE "L1" WITH 4-⅝"ø⅝"X4½" CONCRETE ANCHORS JUST BELOW ROOF DECK AND AT MID-HEIGHT OF PANELS. TYPICAL ALL 90^ CORNERS AS SHOWN.
2. PERIMETER ANGLE VERTICAL LEG UP AT JOIST BEARING WALLS AND VERTICAL LEG DOWN AT JOIST GIRDER BEARING WALLS (SEE SECTIONS FOR SIZES). WELD CORNERS AND BUTT WELD SPLICES TO MAKE CONTINUOUS.
3. WALL BRACE ANGLE 2½"X2½"X3/16"X20'-0" TYPICAL AT EXTERIOR WALLS.
4. "x" DENOTES LOCATION OF BOTTOM CHORD EXTENSIONS.
5. JOISTS AND JOIST GIRDERS SHALL BE DESIGNED FOR THE NET UPLIFT AS SHOWN IN THE DIAGRAMS (ON SHEET S300). VERIFICATION OF EXACT BRIDGING AND BOTTOM CHORD EXTENSION REQUIREMENTS IS THE RESPONSIBILITY OF THE MANUFACTURER.
6. JOIST SEATS AT BEARING WALLS SHALL BE DESIGNED FOR AN UNFACTORED LATERAL ROLLOVER SEISMIC FORCE OF 1,425 LB. OR WIND FORCE OF 1,400 LB. APPLIED AT THE TOP OF THE JOIST AND TRANSFERRED TO THE BEARING SURFACE.
7. WELDED CROSS BRIDGING IN ADDITION TO HORIZONTAL BRIDGING IS REQUIRED WHERE SHOWN BY ><.
8. ALL JOIST SHOES ARE 2½" DEEP, EXCEPT AT LINE 2.8 WHERE THEY ARE 5" DEEP, ALL JOIST GIRDER SHOES ARE 7½" DEEP.
9. ALL ELEVATIONS ARE TO JOIST BEARING UNLESS NOTED OTHERWISE.



KEY PLAN NORTH



JOIST NET UPLIFT DIAGRAM NORTH



JOIST GIRDER NET UPLIFT DIAGRAM NORTH

PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

NO.	DATE	DESCRIPTION	BY
4	2-10-09	RELEASED FOR CONSTRUCTION	STR
3	10-7-08	RELEASED FOR PERMIT	STR
2	8-14-08	RELEASED FOR REVIEW	STR
1	7-21-08	RELEASED FOR REVIEW	STR

REVISIONS

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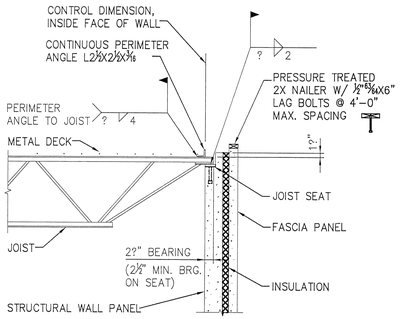
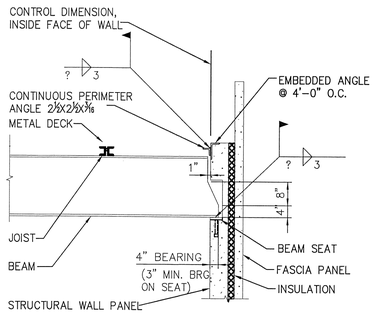
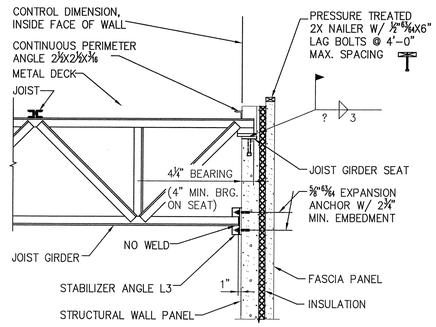
SHEET TITLE	ROOF FRAMING PLAN
DESIGNED JML	JOB NO. 103220
DRAWN JMH	
CHECKED STR	SHEET NO. S300
DATE JUL 08	



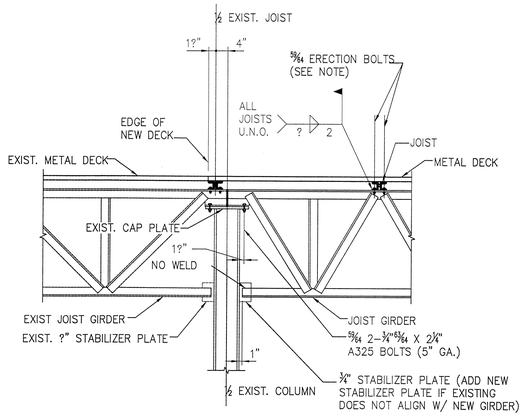
JOIST WALL BEARING SECTION S4 / S300



BEAM WALL BEARING SECTION S11 / S300

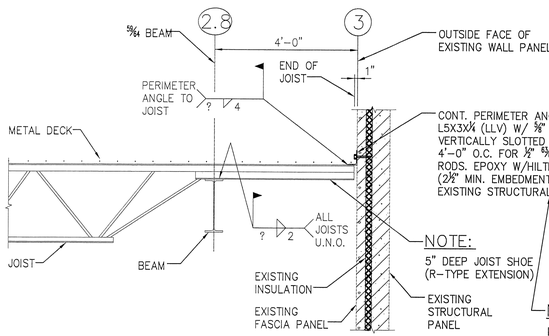


JOIST GIRDER WALL BEARING SECTION S24 / S300



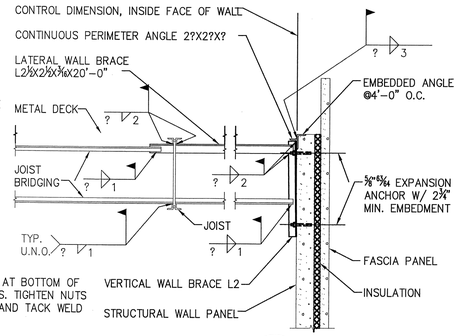
COLUMN–JOIST GIRDER–JOIST CONNECTIONS25 / S300

NOTE: PROVIDE ½"ø⅝"x1¾" A307 ERECTION BOLTS FOR ALL JOISTS IN BAYS 40 FEET OR MORE. GIRDER DESIGN SHALL ACCOUNT FOR BOLT HOLES ASSUMING BOLTS WILL NOT BE INSTALLED.

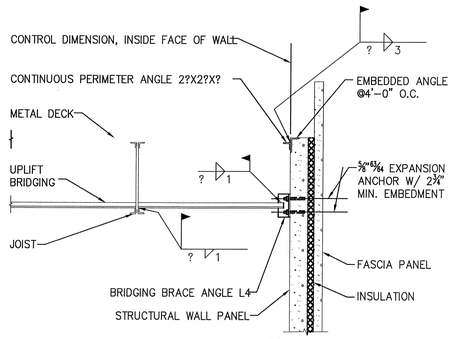


BEAM–JOIST CONNECTION S48 / S300

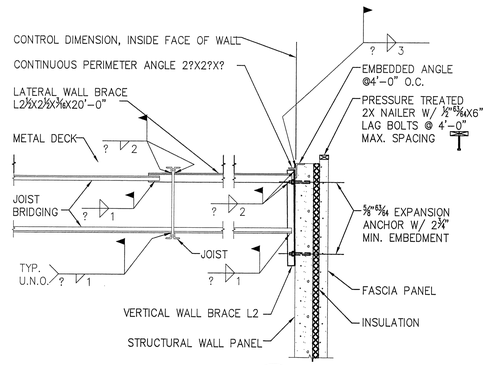
NOTE: INSTALL RODS AT BOTTOM OF SLOTTED HOLES. TIGHTEN NUTS FINGER TIGHT AND TACK WELD NUT TO ROD.



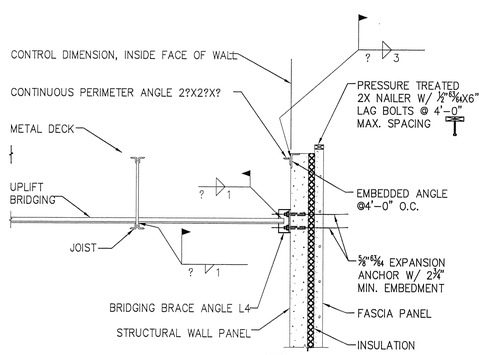
WALL BRACE SECTION S51 / S300



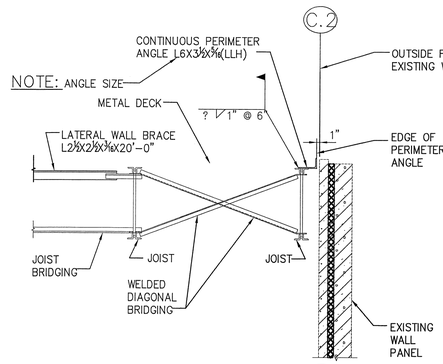
UPLIFT BRIDGING SECTION S53 / S300



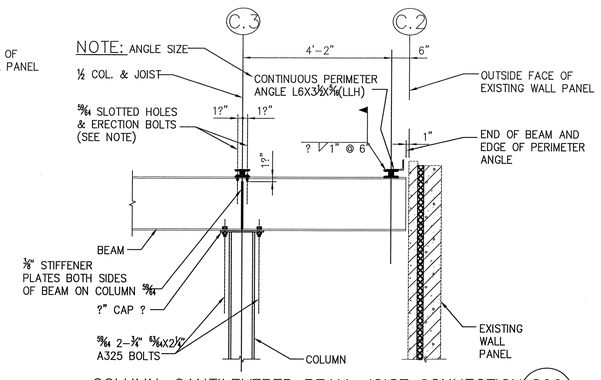
WALL BRACE SECTION S57 / S300



UPLIFT BRIDGING SECTION S59 / S300

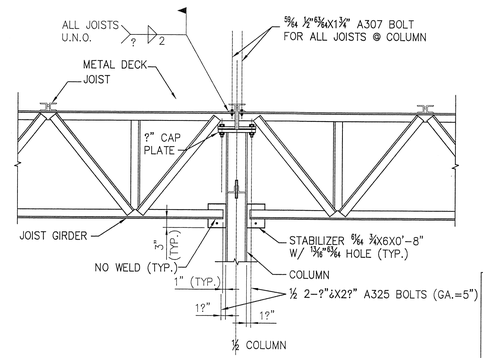


DIAGONAL BRIDGING SECTION S65 / S300



COLUMN–CANTILEVERED BEAM–JOIST CONNECTION S66 / S300

NOTE: PROVIDE ½"ø⅝"x1¾" A307 ERECTION BOLTS FOR ALL JOISTS AT OR ADJACENT TO COLUMN.



COLUMN–JOIST GIRDER–JOIST CONNECTION S72 / S300

NOTE: PROVIDE SLOTTED HOLES IN JOIST & JOIST GIRDER SHOES @ COLUMN

PROJECT RECORD 09.14.09

NO.	DATE	DESCRIPTION	BY
4	2-10-09	RELEASED FOR CONSTRUCTION	STR
3	10-7-08	RELEASED FOR PERMIT	STR
2	8-14-08	RELEASED FOR REVIEW	STR
1	7-21-08	RELEASED FOR REVIEW	STR

REVISIONS

CON/STEEL® TILT-UP SYSTEMS (937) 256-1996

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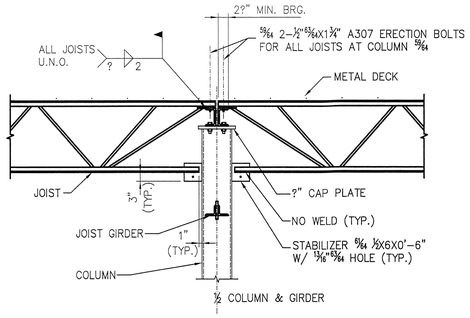
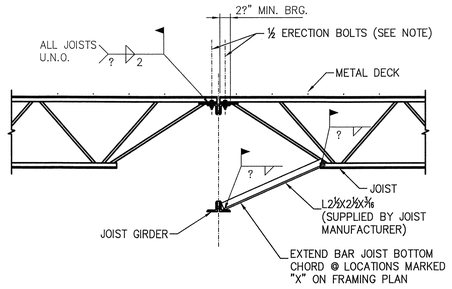
CON/STEEL ALLIANCE

SHEET TITLE: FRAMING DETAILS

DESIGNED	JML
DRAWN	JMH
CHECKED	STR
DATE	JUL. 08
JOB NO.	103220
SHEET NO.	S301

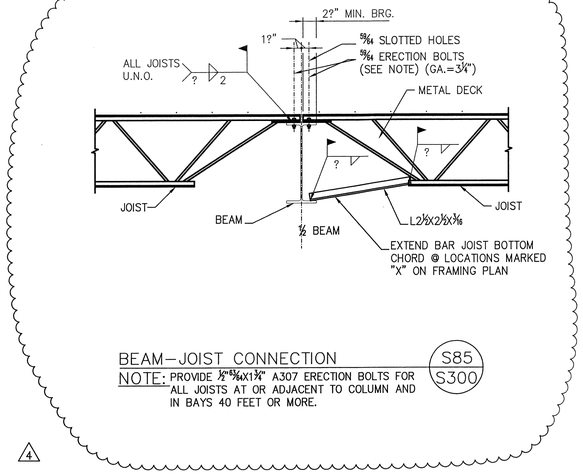


COLUMN–JOIST–JOIST GIRDER CONNECTION S82 / S300



JOIST GIRDER–BOTTOM CHORD BRACE S83 / S300

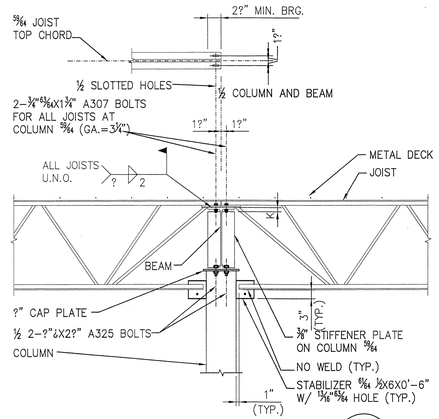
NOTE: PROVIDE ½"ø⅝x1¾" A307 ERECTION BOLTS FOR ALL JOISTS IN BAYS 40 FEET OR MORE. GIRDER DESIGN SHALL ACCOUNT FOR BOLT HOLES ASSUMING BOLTS WILL NOT BE INSTALLED.



4

BEAM–JOIST CONNECTION S85 / S300

NOTE: PROVIDE ½"ø⅝x1¾" A307 ERECTION BOLTS FOR ALL JOISTS AT OR ADJACENT TO COLUMN AND IN BAYS 40 FEET OR MORE.



COLUMN–BEAM–JOIST CONNECTION S86 / S300

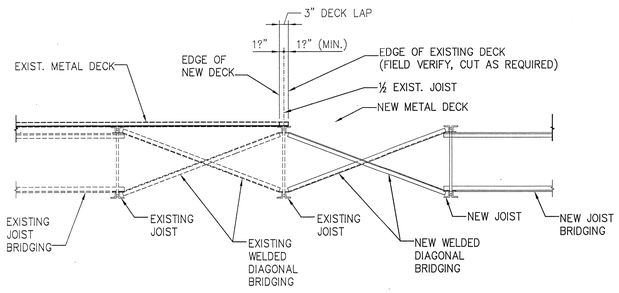
SCALE: ?" = 1'-0"

JOIST GIRDER–BOTTOM CHORD BRACE AFTER EXPANSION S87 / S300

NOTE: PROVIDE ½"ø⅝x1¾" A307 ERECTION BOLTS FOR ALL JOISTS IN BAYS 40 FEET OR MORE.

COLUMN–JOIST–JOIST GIRDER CONNECTION S88 / S300

BEAM BEARING ON EXISTING WALL PANEL S90 / S300



DIAGONAL BRIDGING SECTION S91 / S300

NO.	DATE	DESCRIPTION	BY
4	2-10-09	RELEASED FOR CONSTRUCTION	STR
3	10-7-08	RELEASED FOR PERMIT	STR
2	8-14-08	RELEASED FOR REVIEW	STR
1	7-21-08	RELEASED FOR REVIEW	STR

REVISIONS

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MADISON, WISCONSIN

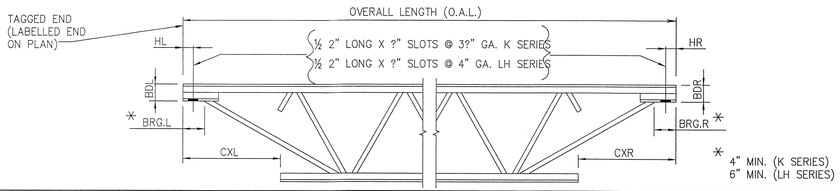
PROJECT RECORD
09.14.09

CON/STEEL ALLIANCE

SHEET TITLE: FRAMING DETAILS

DESIGNED	JML	JOB NO.	103220
DRAWN	JMH		
CHECKED	STR	SHEET NO.	S302
DATE	JUL 08		

BAR JOIST SCHEDULE

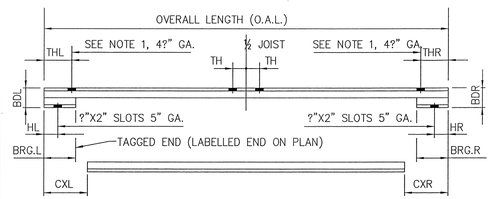


HORIZONTAL DIM. DETAILING (±) IN	HORIZONTAL DIM. DETAILING C/C FT IN	HORIZONTAL DIM. DETAILING (±) IN	QTY.	MARK	TYPE	O.A.L. FT IN	CXL IN	CXR IN	BRG.L IN	BRG.R IN	BDL IN	BDR IN	HL IN	HR IN	NET UPLIFT (SEE NOTE 3)
+2 7/8	38 - 2 3/4	- 1/8	1	J1	28K8	38 - 5 1/2	—	—	4	4	2 1/2	2 1/2	—	—	111 PLF
+2 7/8	38 - 2 3/4	- 1/8	1	J2	28K8	38 - 5 1/2	—	—	4	4	2 1/2	2 1/2	—	—	SEE NOTE 4
+2 7/8	38 - 2 3/4	- 1/8	17	J3	28K8	38 - 5 1/2	—	—	4	4	2 1/2	2 1/2	—	—	SEE NOTE 5
+2 7/8	38 - 2 3/4	- 1/8	2	J4	28K8	38 - 5 1/2	—	5 7/8	4	4	2 1/2	2 1/2	—	2 1/8	SEE NOTE 5
- 1/8	42 - 0	- 1/8	1	J5	30K9	41 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	111 PLF
- 1/8	42 - 0	- 1/8	1	J6	30K9	41 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	100 PLF
- 1/8	42 - 0	- 1/8	14	J7	30K9	41 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	86 PLF
- 1/8	42 - 0	- 1/8	2	J8	30K9	41 - 11 3/4	5 7/8	5 7/8	4	4	2 1/2	2 1/2	2 1/8	2 1/8	86 PLF
- 1/8	48 - 0	- 1/8	1	J9	30K11	47 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	111 PLF
- 1/8	48 - 0	- 1/8	1	J10	30K11	47 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	100 PLF
- 1/8	48 - 0	- 1/8	17	J11	30K11	47 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	86 PLF
- 1/8	48 - 0	- 1/8	2	J12	30K11	47 - 11 3/4	5 7/8	5 7/8	4	4	2 1/2	2 1/2	2 1/8	2 1/8	86 PLF
+2 7/8	28 - 4	- 1/8	1	J13	24KSP01	28 - 6 3/4	—	—	4	4	2 1/2	2 1/2	—	—	33 PLF
+2 7/8	28 - 4	- 1/8	1	J14	24KCS2	28 - 6 3/4	—	3 7/8	4	4	2 1/2	2 1/2	—	1 5/8	69 PLF
+2 7/8	28 - 4	- 1/8	3	J15	24KCS2	28 - 6 3/4	—	—	4	4	2 1/2	2 1/2	—	—	69 PLF
- 1/8	18 - 10	+47	1	J16	24KSP02	22 - 8 7/8	—	—	4	51	2 1/2	5	—	—	33 PLF
- 1/8	18 - 10	+47	1	J17	24KSP03	22 - 8 7/8	3 7/8	—	4	51	2 1/2	5	1 5/8	—	69 PLF
- 1/8	18 - 10	+47	3	J18	24KSP03	22 - 8 7/8	—	—	4	51	2 1/2	5	—	—	69 PLF
- 1/8	42 - 0	- 1/8	3	J19	30KCS4	41 - 11 3/4	—	—	4	4	2 1/2	2 1/2	2 1/8	2 1/8	86 PLF

NOTE:
1. TAGGED END AS SHOWN ON PLAN CORRESPONDS TO LEFT END AS SHOWN IN SCHEDULE.
2. VERIFY BRG.L AND BRG.R WITH JOIST GIRDER TOP CHORD WIDTH.
3. ALLOWABLE MATERIAL STRESS INCREASES ARE NOT PERMITTED PER IBC SECTION 1605.3.1.
4. 111 PLF FROM 0'-0" TO 13'-3" FROM LEFT (TAGGED) END, 100 PLF ON REMAINDER OF JOIST.
5. 111 PLF FROM 0'-0" TO 13'-3" FROM LEFT (TAGGED) END, 86 PLF ON REMAINDER OF JOIST.

JOIST GIRDER SCHEDULE

MARK	JOIST SPACING FT IN		FT IN	FT IN	FT IN	HORIZONTAL DIM. DETAILING (±) IN	C/C FT IN	(±) IN
G1	6 - 7	4 SPACES @	6 - 0	= 24 - 0	5 - 11 3/4	+4 1/4	36 - 2 3/4	- 1/4
G2	5 - 11 3/4	6 SPACES @	6 - 0	= 36 - 0	5 - 11 3/4	- 1/4	48 - 0	- 1/4
G3	6 - 7	4 SPACES @	6 - 0	= 24 - 0	5 - 11 3/4	+4 1/4	36 - 2 3/4	- 1/4
G4	5 - 11 3/4	6 SPACES @	6 - 0	= 36 - 0	5 - 11 3/4	- 1/4	48 - 0	- 1/4
G5	5 - 11 3/4	6 SPACES @	6 - 0	= 36 - 0	5 - 11 3/4	- 1/4	48 - 0	- 1/4



NOTES:
1. USE $\frac{3}{8}$"Ø$\frac{5}{16}$ HOLES FOR THL & THR = 1$\frac{3}{8}$" & 1$\frac{3}{8}$"Ø$\frac{5}{16}$ HOLES FOR THL & THR = 1$\frac{3}{4}$".
2. TH HOLES SHALL BE AT EACH JOIST WHEN SHOWN IN SCHEDULE.
3. USE $\frac{3}{8}$"Ø$\frac{5}{16}$ HOLES FOR TH = 1$\frac{3}{8}$" & 1$\frac{3}{8}$"Ø$\frac{5}{16}$ HOLES FOR TH = 2".
4. TH HOLES SHALL BE AT 4$\frac{1}{2}$" GA. NEAR SIDE, FAR SIDE OR EACH SIDE AS NOTED IN TABLE.
5. GIRDER WEIGHT HAS BEEN ADDED TO KIP LOAD.

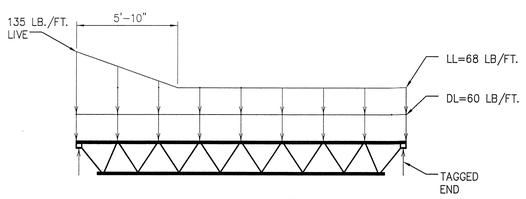
QTY.	MARK	TYPE	O.A.L. FT IN	CXL IN	CXR IN	BRG.L IN	BRG.R IN	BDL IN	BDR IN	HL IN	HR IN	THL IN	TH IN	THR IN	COMMENTS (SEE COMMENT 3)
1	G1	40G8N13.6K	36 - 6 3/4	5 1/4	5 3/4	6	7 3/4	7 1/2	7 1/2	—	6 1/4	—	1 5/8 NS	1 3/8	SEE NOTE 4
2	G2	40G8N13.6K	47 - 11 1/2	5 3/4	5 3/4	7 3/4	7 3/4	7 1/2	7 1/2	6 1/4	6 1/4	1 3/8	1 5/8 NS	1 3/8	457 PLF NET UPLIFT
1	G3	48G8N15.2K	36 - 6 3/4	5 1/4	5 3/4	6	7 3/4	7 1/2	7 1/2	—	6 1/4	—	1 5/8 ES	1 3/8	SEE NOTE 5
1	G4	58G8NSP01	47 - 11 1/2	5 3/4	5 3/4	7 3/4	7 3/4	7 1/2	7 1/2	6 1/4	6 1/4	1 3/8	1 5/8 ES	1 3/8	450 PLF NET UPLIFT
1	G5	58G8N15.2K	47 - 11 1/2	5 3/4	5 3/4	7 3/4	7 3/4	7 1/2	7 1/2	6 1/4	6 1/4	1 3/8	1 5/8 ES	1 3/8	450 PLF NET UPLIFT

COMMENTS:
1. TAGGED END AS SHOWN ON PLAN CORRESPONDS TO LEFT END AS SHOWN IN SCHEDULE.
2. "NS" DENOTES NEAR SIDE, "FS" DENOTES FAR SIDE, "ES" DENOTES EACH SIDE.
3. ALLOWABLE MATERIAL STRESS INCREASES ARE NOT PERMITTED PER IBC SECTION 1605.3.1.
4. 642 PLF NET UPLIFT FROM 0'-0" TO 26'-6" FROM LEFT (TAGGED) END, 457 PLF ON REMAINDER OF GIRDER.
5. 720 PLF NET UPLIFT FROM 0'-0" TO 26'-6" FROM LEFT (TAGGED) END, 450 PLF ON REMAINDER OF GIRDER.

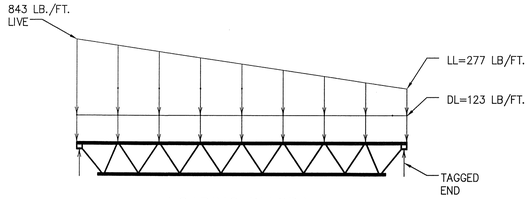
ROOF BRIDGING MATERIAL LIST

NO. REQ'D	MARK	SIZE	SPACE	JOIST SIZE	JOIST SIZE	OTHERS	LENGTH	A	B	G	MAKE LIKE
34	UPLIFT	1 1/4 X 1 1/4 X 7/64					20'-0"				
15	UPLIFT	1 1/2 X 1 1/2 X 7/64					20'-0"				
95	HORIZ.	1 1/4 X 1 1/4 X 7/64					20'-0"				
43	HORIZ.	1 1/2 X 1 1/2 X 7/64					20'-0"				
6	WELDED	1 1/4 X 1 1/4 X 7/64	6'- 4 "	28	28		6'-9"				
6	WELDED	1 1/4 X 1 1/4 X 7/64	6'- 4 "	30	30		6'-10"				
6	WELDED	1 1/2 X 1 1/2 X 7/64	6'- 4 "	30	30		6'-10"				

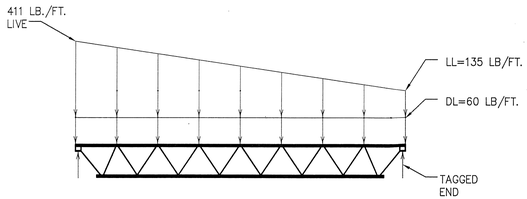
NOTE: VERIFICATION OF LENGTHS AND QUANTITIES IS THE RESPONSIBILITY OF THE JOIST MANUFACTURER (SEE FRAMING NOTES).



LOADING DIAGRAM
24KSP01



LOADING DIAGRAM
24KSP03



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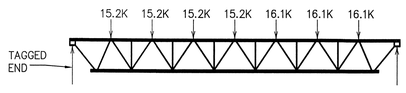

LOADING DIAGRAM
58G8NSP01

NO.	DATE	DESCRIPTION	BY
3	2-10-09	RELEASED FOR CONSTRUCTION	STR
2	10-7-08	RELEASED FOR PERMIT	STR
1	8-14-08	RELEASED FOR REVIEW	STR

REVISIONS

CONSTEEL® TILT-UP SYSTEMS (937) 226-9966

RUEDEBUSCH DEVELOPMENT & CONSTRUCTION
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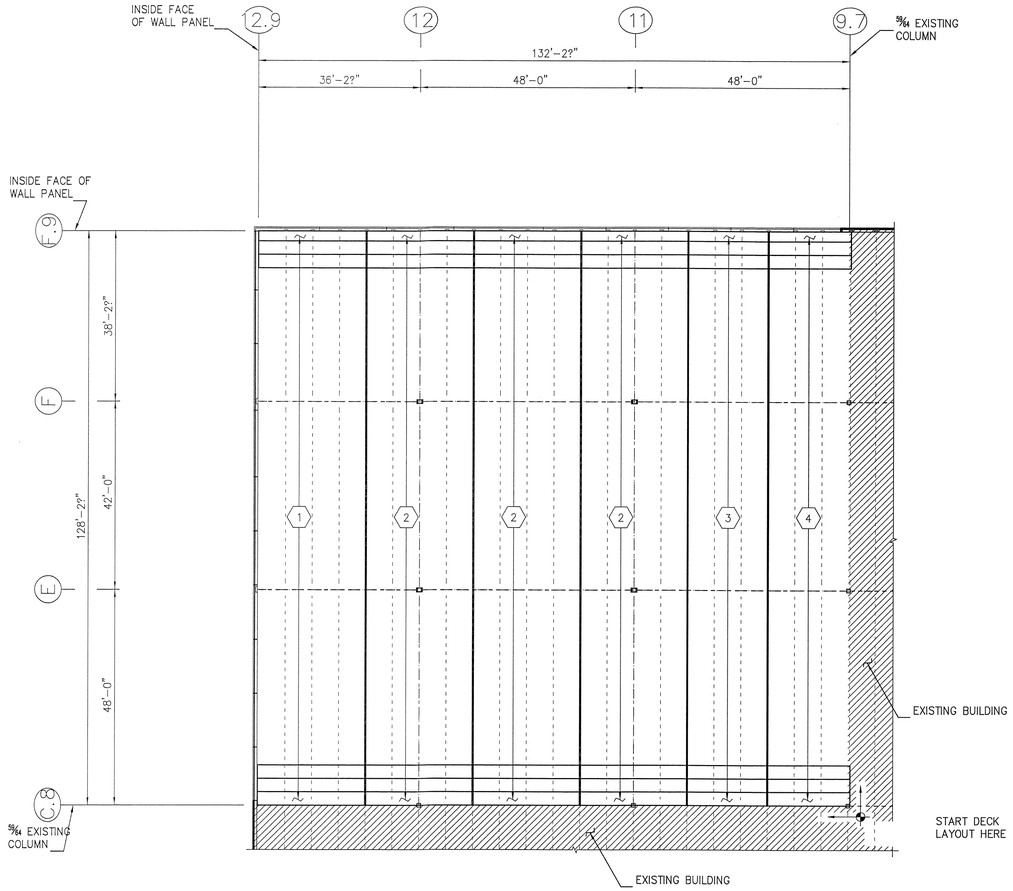
MADISON, WISCONSIN

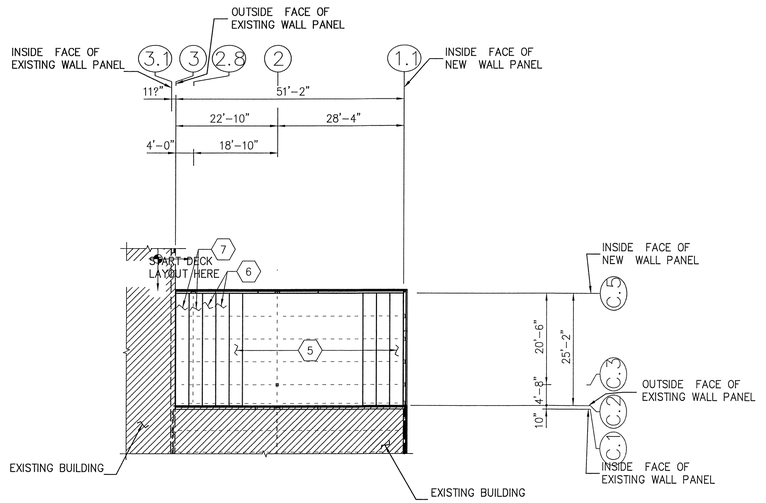
PROJECT RECORD
09.14.09

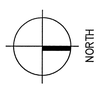
CON/STEEL ALLIANCE

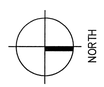
SHEET TITLE: JOIST AND JOIST GIRDER SCHEDULES

DESIGNED	JML	JOB NO.	103220
DRAWN	JMH		
CHECKED	STR	SHEET NO.	S303
DATE	AUG. 08		



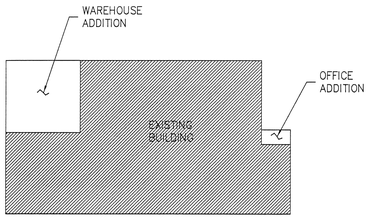


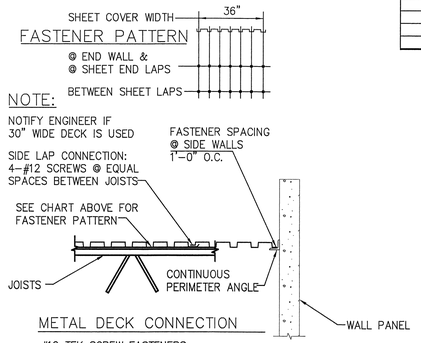




MATERIAL LIST

1-1/2" TYPE B METAL ROOF DECK 18 GA. 36" COVERAGE CONTRACTOR VERIFY PAINT COLOR			1-1/2" TYPE B METAL ROOF DECK 20 GA. 36" COVERAGE CONTRACTOR VERIFY PAINT COLOR			1-1/2" TYPE B METAL ROOF DECK 22 GA. 36" COVERAGE CONTRACTOR VERIFY PAINT COLOR		
MARK	NO. PIECES	LENGTH	MARK	NO. PIECES	LENGTH	MARK	NO. PIECES	LENGTH
7	2	25'- 0 "	6	2	25'- 0 "	1	43	24'- 4 1/2"
						2	129	24'- 3 "
						3	43	18'- 3 "
						4	43	18'- 7 "
						5	14	25'- 0 "





PROJECT RECORD
09.14.09

CON/STEEL ALLIANCE

REVISIONS

NO.	DATE	DESCRIPTION	BY
3	2-10-09	RELEASED FOR CONSTRUCTION	STR
2	10-7-08	RELEASED FOR PERMIT	STR
1	8-14-08	RELEASED FOR REVIEW	STR

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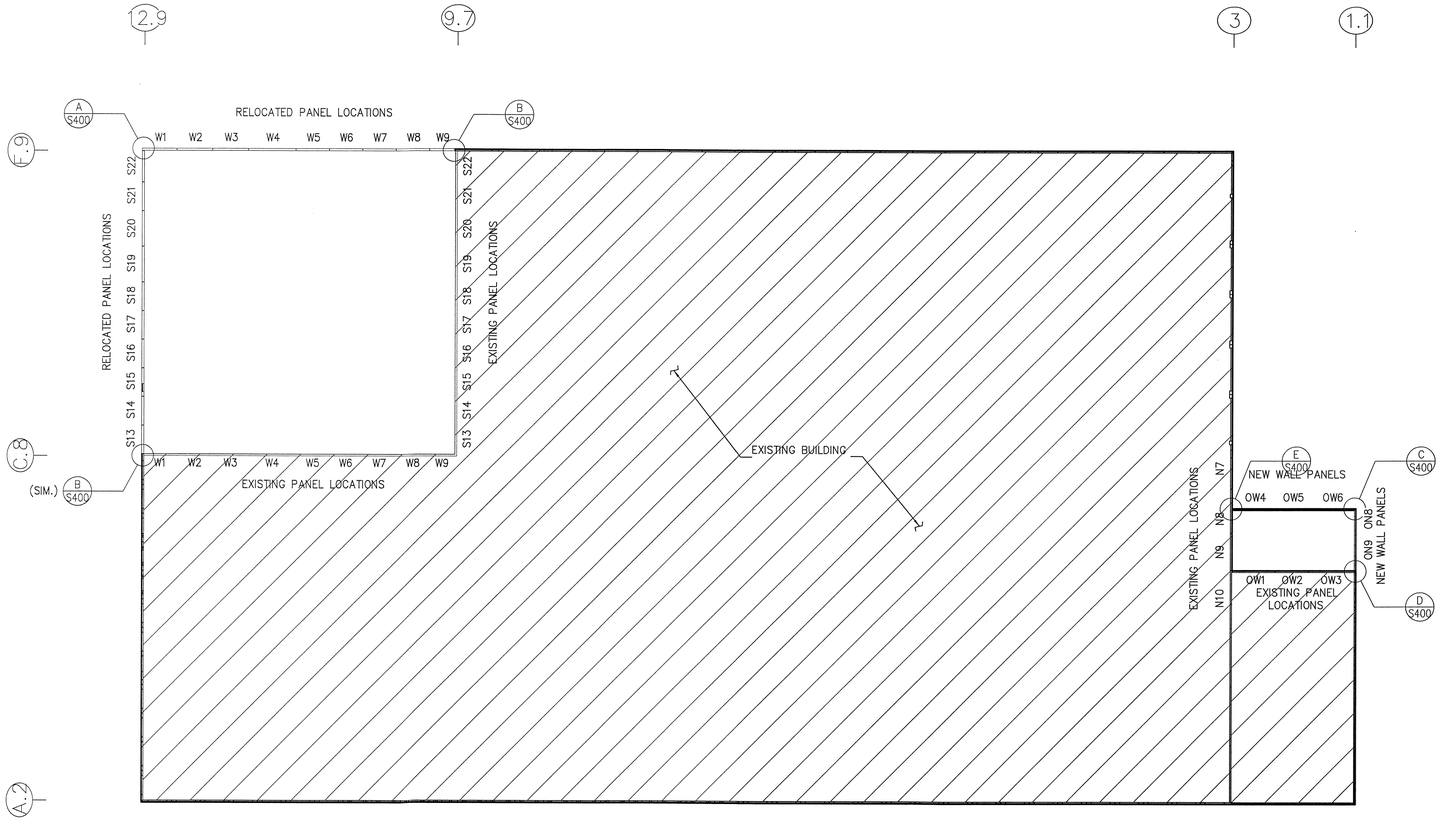
3901 HANSON ROAD
MADISON, WISCONSIN

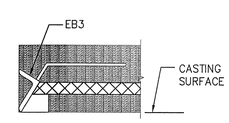
SHEET TITLE
ROOF DECK LAYOUT

DESIGNED JML	JOB NO. 103220
DRAWN JMH	
CHECKED STR	SHEET NO. S304
DATE AUG. 08	

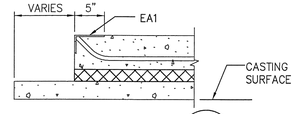


PANEL LAYOUT NOTES

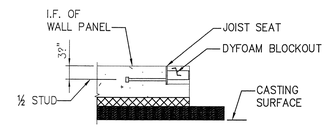
1. ALL PANELS SHOWN OUTSIDE FACE DOWN.
2. FOR REINFORCING STEEL PLACEMENT SEE PANEL REINFORCING SHEET(S).
3. FOR PANEL LIFTING INSERT LOCATIONS SEE LIFTING INSERT SHEET(S).
4. CHLORIDE BASED ADMIXTURES ARE PROHIBITED IN CONCRETE USED FOR WALL PANELS.
5. TEMPORARY BRACING SHALL BE PROVIDED FOR WALL PANELS IN THE ERECTED POSITION. INSTALLATION OF WALL BRACES SHALL MEET ALL REQUIREMENTS OF THE BRACE SUPPLIER.
6. WHEN CUTTING FUTURE OPENINGS AT THE LOCATIONS SHOWN, THE MAXIMUM OVERCUT PERMITTED AT THE CORNERS IS 5". THESE OVERCUTS SHALL BE GROUTED SOLID WITH NON-SHRINK, NON-METALLIC GROUT UPON THE REMOVAL OF THE CUT OUT SECTION.



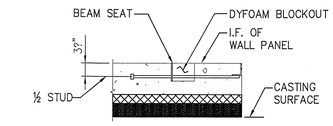
SECTION 1 / S401



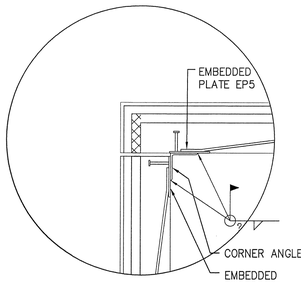
SECTION 2 / S401



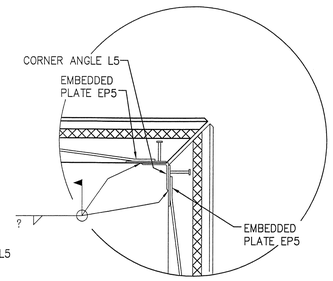
TYPICAL JOIST SEAT



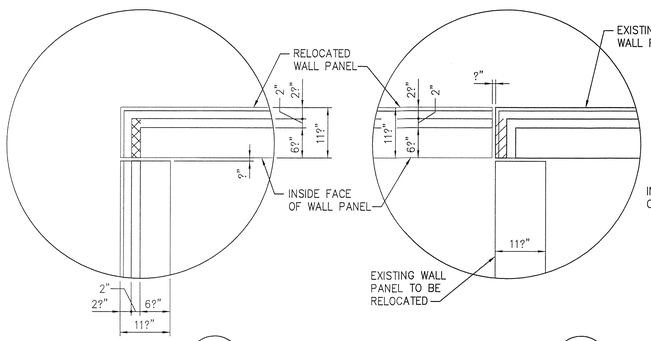
TYPICAL BEAM SEAT



SECTION 3 / S402

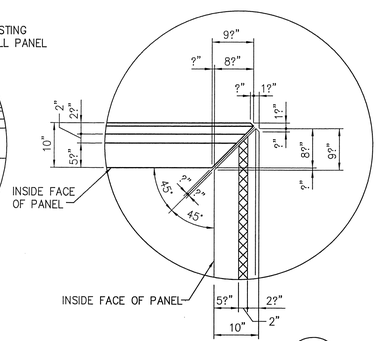


SECTION 4 / S401

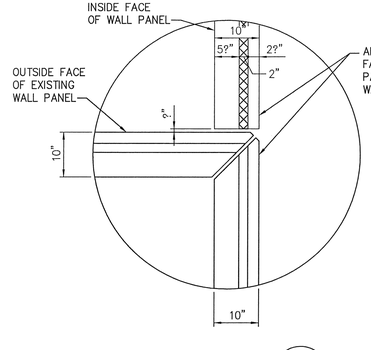


DETAIL A / S400

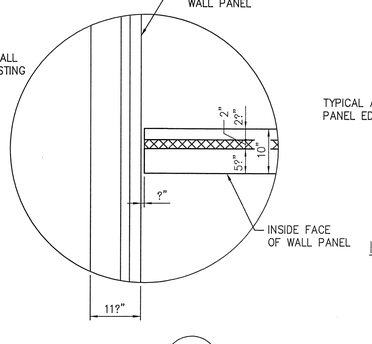
DETAIL B / S400



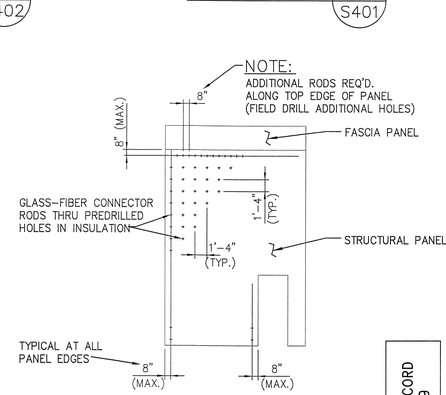
DETAIL C / S400



DETAIL D / S400



DETAIL E / S400



NOTE:
FOLLOW THERMOMASS INSTALLATION GUIDELINES TO ENSURE PROPER CONCRETE CONSOLIDATION AROUND CONNECTOR RODS.

INSULATED PANEL CONNECTOR LAYOUT

PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

NO.	DATE	DESCRIPTION	BY
		REVISIONS	
2	2-10-09	RELEASED FOR CONSTRUCTION	STR
1	8-14-08	RELEASED FOR REVIEW	STR

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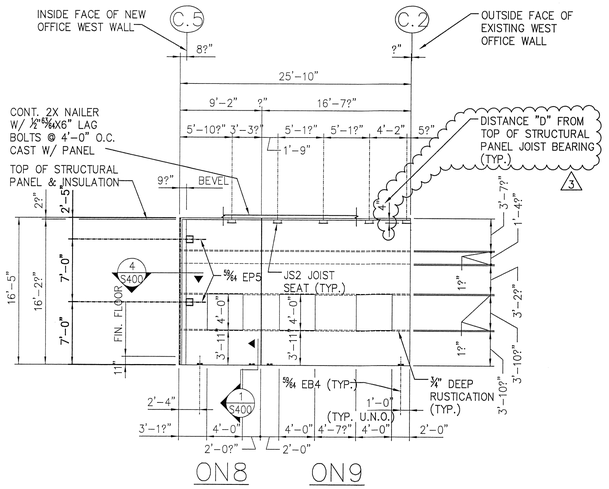
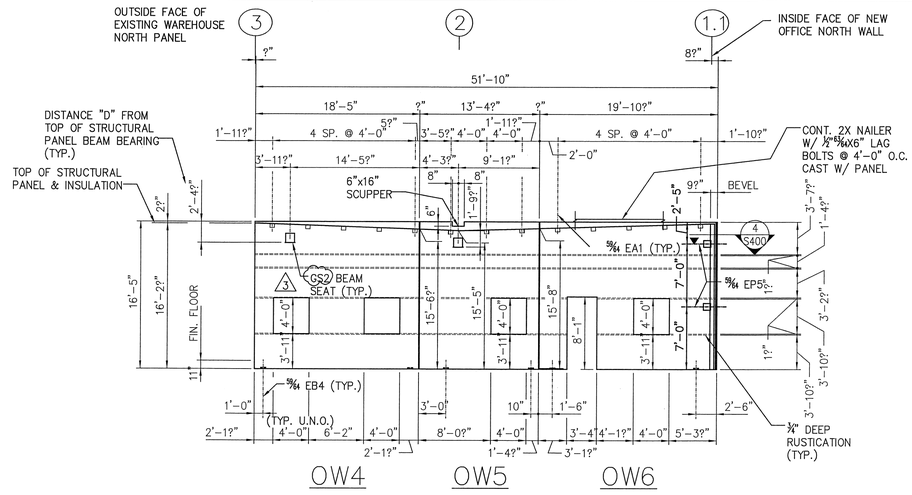
SHEET TITLE	KEY PLAN
DESIGNED JML	JOB NO. 103220
DRAWN JMH	
CHECKED STR	SHEET NO. S400
DATE AUG. 08	



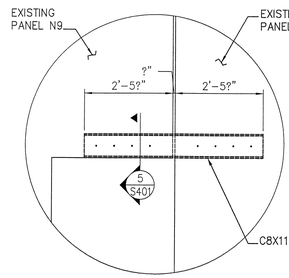
OFFICE NORTH WALL

5?" THICK SMOOTH FINISH
2" THICK INSULATION
2?" THICK FASCIA PANEL
?" DEEP RUSTICATION

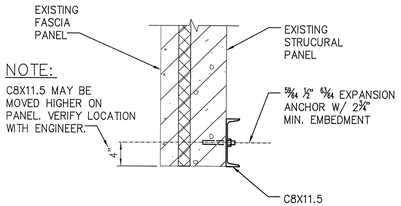


OFFICE WEST WALL

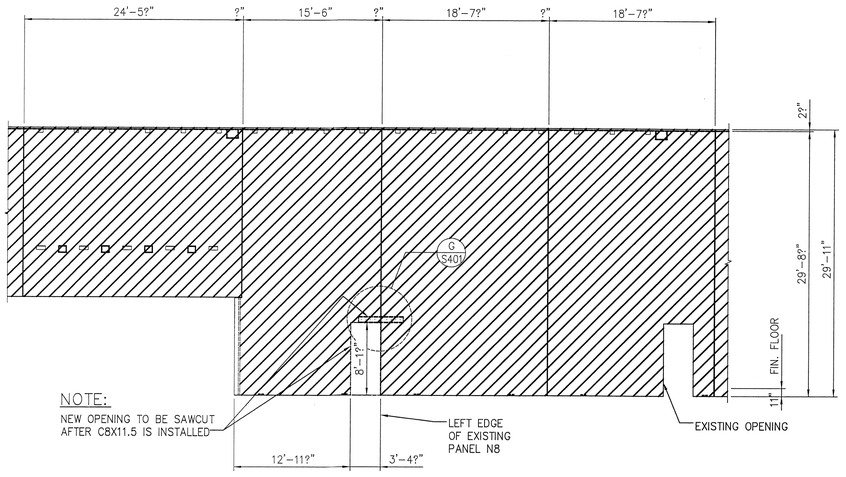
5?" THICK SMOOTH FINISH
2" THICK INSULATION
2?" THICK FASCIA PANEL
?" DEEP RUSTICATION



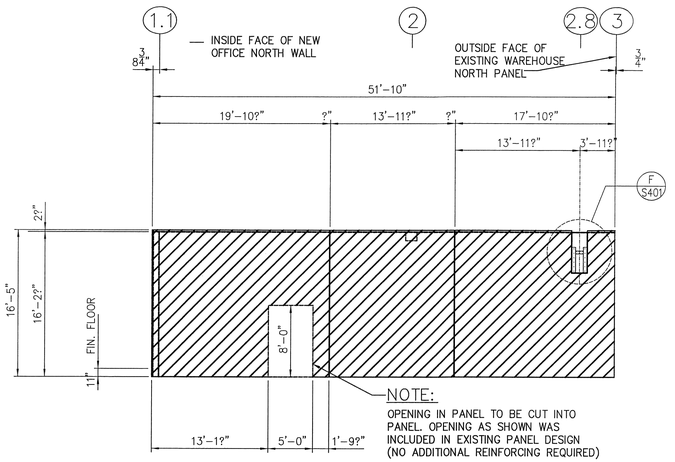
DETAIL G S401



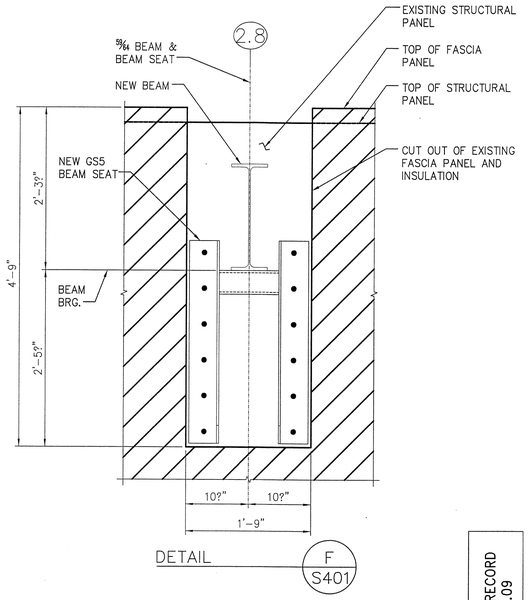
SECTION 5 S401



EXISTING OFFICE NORTH WALL VIEWED FROM NORTH



EXISTING OFFICE WEST WALL VIEWED FROM WEST



DETAIL F S401

PROJECT RECORD
09.14.09

NO.	DATE	DESCRIPTION	BY
2	2-10-09	RELEASED FOR CONSTRUCTION	STR
1	8-14-08	RELEASED FOR REVIEW	STR

REVISIONS

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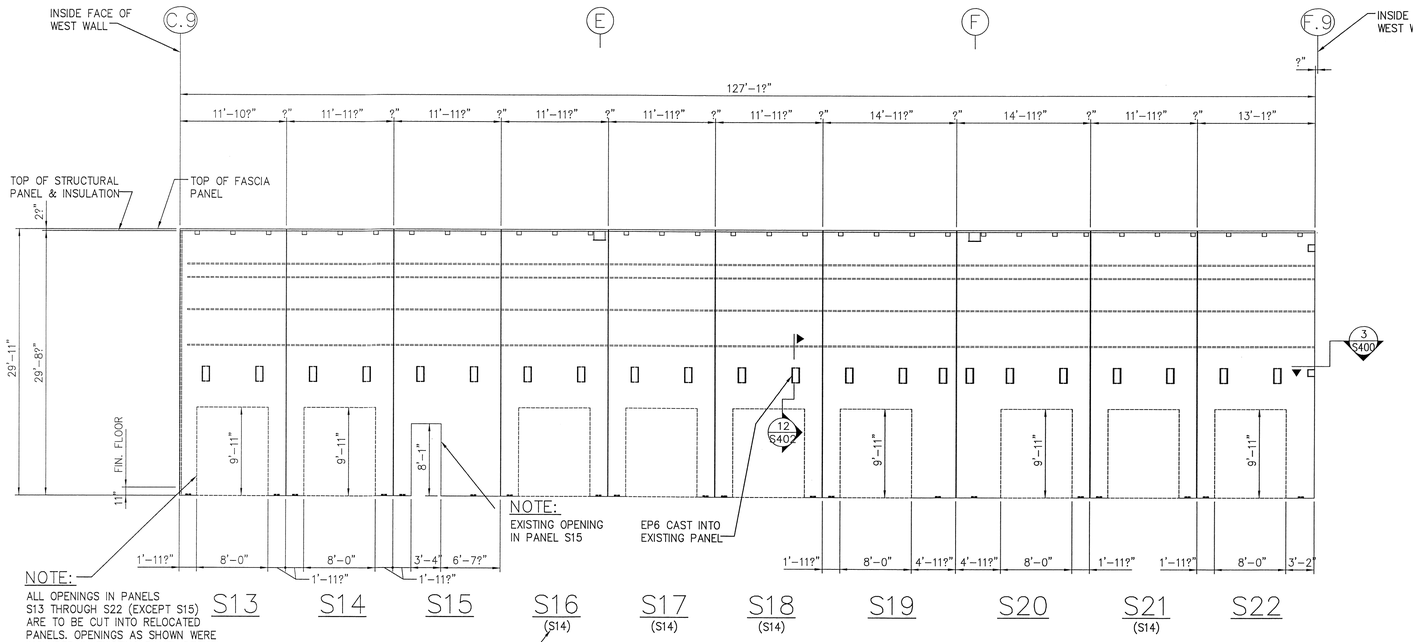
PACKAGE DISTRIBUTION CENTER ADDITIONS
3901 HANSON ROAD
MADISON, WISCONSIN

SHEET TITLE
WALL PANEL LAYOUT

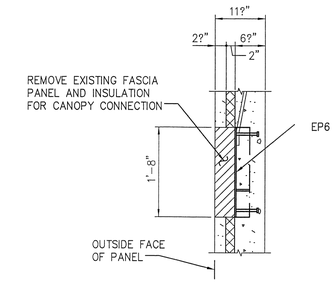
DESIGNED	JML	JOB NO.	103220
DRAWN	JMH		
CHECKED	STR	SHEET NO.	S401
DATE	AUG. 08		

CON/STEEL ALLIANCE

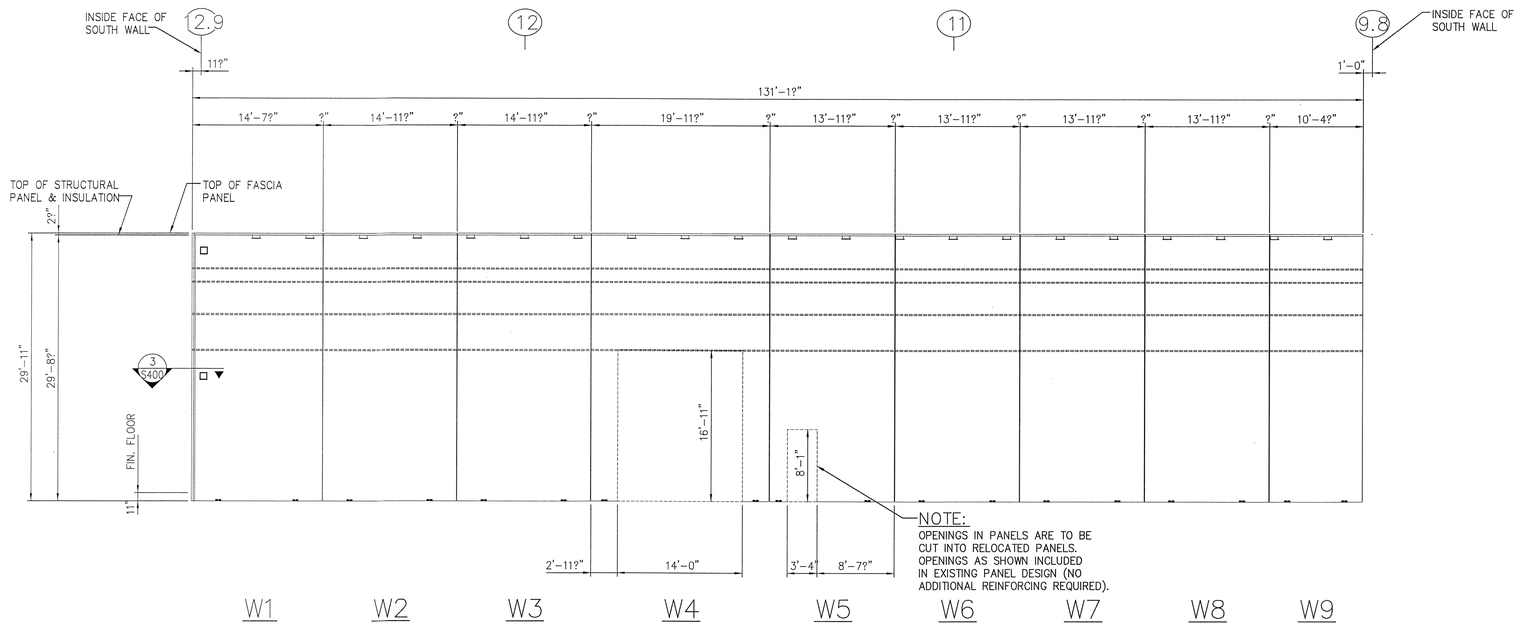


RELOCATED SOUTH WALL

6¾" THICK STRUCTURAL PANEL
2" THICK INSULATION
2½" THICK FASCIA PANEL
¾" DEEP RUSTICATION



SECTION 12 / S402



RELOCATED WEST WALL

6¾" THICK STRUCTURAL PANEL
2" THICK INSULATION
2½" THICK FASCIA PANEL
¾" DEEP RUSTICATION

NO.	DATE	DESCRIPTION	BY
1	2-10-09	RELEASED FOR CONSTRUCTION	STR

REVISIONS

CONSTEEL® TILT-UP SYSTEMS (937) 256-6966

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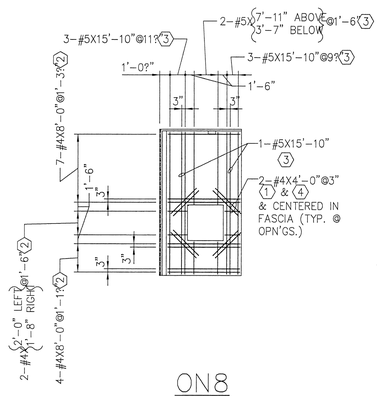
PACKAGE DISTRIBUTION CENTER ADDITIONS
3901 HANSON ROAD
MADISON, WISCONSIN

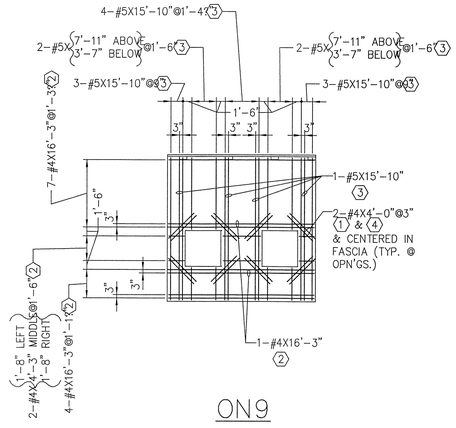
PROJECT RECORD
09.14.09

CONSTEEL ALLIANCE

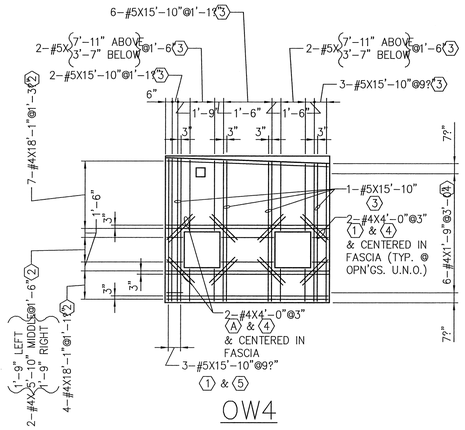
SHEET TITLE	RELOCATED WALL PANEL LAYOUT
DESIGNED JML	JOB NO.
DRAWN JMH	103220
CHECKED STR	SHEET NO.
DATE AUG. 08	S402



ON8

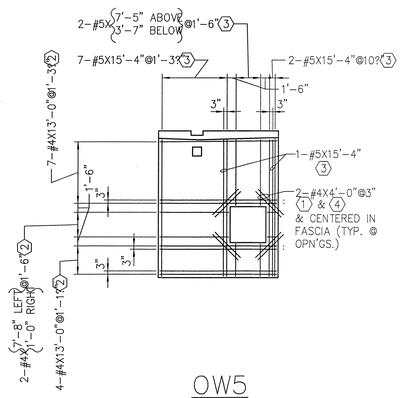


ON9



OW4

NOTE: FIELD CUT BARS TO FIT TOP EDGE OF FORM

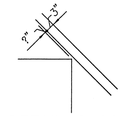


OW5

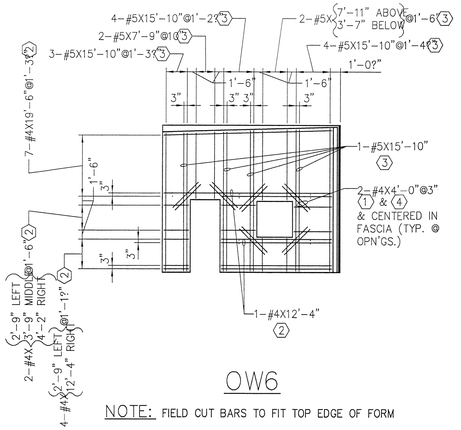
NOTE: FIELD CUT BARS TO FIT TOP EDGE OF FORM

PANEL REINFORCING NOTES

1. MAINTAIN 3" CLEAR FOR ALL REINFORCING BARS PARALLEL TO FORMED EDGES OR PANEL JOINTS AND 6" CLEAR FOR ALL BARS PARALLEL TO FUTURE OPENINGS UNLESS SHOWN OTHERWISE. ADDITIONAL REINFORCING BARS AT FORMED OR FUTURE OPENING SHALL BE LOCATED AS SHOWN.
2. MAINTAIN 2" CLEAR BETWEEN ENDS OF ALL BARS AND FORMED EDGES.
3. THE CLEAR DISTANCE BETWEEN PARALLEL BARS IN A LEVEL SHALL NOT BE LESS THAN 2".
4. THE SYMBOL ⟨L⟩ DENOTES LEVEL OF REINFORCING BARS.
5. WHERE THE VERTICAL REINFORCING BARS ARE REQUIRED IN LEVELS ⟨1⟩, ⟨3⟩ AND ⟨5⟩, LEVEL ⟨5⟩ BARS SHALL BE PLACED DIRECTLY ABOVE LEVEL ⟨1⟩ BARS. LEVEL ⟨3⟩ BARS SHALL BE PLACED TO CLEAR BARS ON LEVEL ⟨1⟩ OR LEVEL ⟨5⟩ BY A MINIMUM OF 2" HORIZONTALLY.
6. BARS SHALL NOT BE SPLICED UNLESS ENGINEER IS NOTIFIED.
7. REINFORCING BARS MAY BE FIELD CUT TO MAINTAIN MINIMUM CLEARANCE OF ALL JOIST SEATS.
8. LEVEL ⟨2⟩ REINFORCING BARS SHALL BE SUPPORTED BY CHAIRS 4'-0" C/C MAXIMUM EACH WAY. ALL OTHER LEVELS SHALL BE SUPPORTED BY AND TIED TO LEVEL ⟨2⟩ BARS UNLESS NOTED.
9. HORIZONTAL AND VERTICAL REINFORCING BARS SHALL BE SECURELY TIED AT 50% OF THEIR INTERSECTIONS. TIES ARE TO BE DISTRIBUTED UNIFORMLY THROUGHOUT REINFORCING MAT.
10. ALL VERTICAL BARS FOR WHICH THE SPACING IS NOT GIVEN SHALL BE EQUALLY SPACED.
11. BARS LISTED TOGETHER IN MULTIPLE LEVELS ARE THE NUMBER OF BARS IN EACH LEVEL. (I.E. 4–#5 ⟨1⟩ & ⟨5⟩ = 8–#5 TOTAL.)

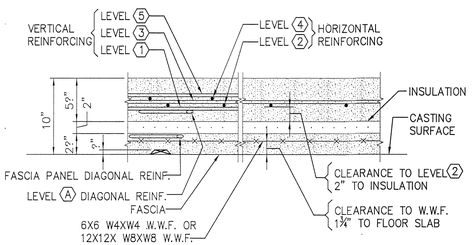


DETAIL @ CORNER OF OPENING



OW6

NOTE: FIELD CUT BARS TO FIT TOP EDGE OF FORM



SMOOTH FINISH W/ ?" RUSTICATION

(REQUIRES 2" HIGH CHAIRS TO LEVEL 2)
(REQUIRES 1?" HIGH CHAIRS TO W.W.F.)

TYPICAL FOR OFFICE PANELS

NO.	DATE	DESCRIPTION	BY
2	2-10-09	RELEASED FOR CONSTRUCTION	STR
1	8-14-08	RELEASED FOR REVIEW	STR

REVISIONS

PROJECT RECORD
09.14.09

CON/STEEL ALLIANCE

CONSTEEL® TILT-UP SYSTEMS (937) 256-3996

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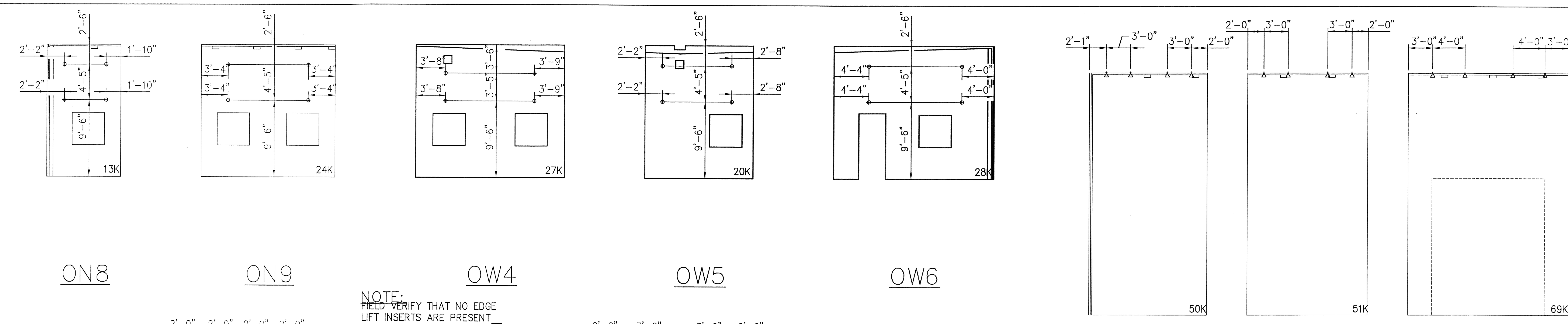
3901 HANSON ROAD
MADISON, WISCONSIN

SHEET TITLE	WALL PANEL REINFORCING
DESIGNED JML	JOB NO. 103220
DRAWN HMW	
CHECKED STR	SHEET NO. S500
DATE AUG. 08	



ON8 ON9 OW4 OW5 OW6

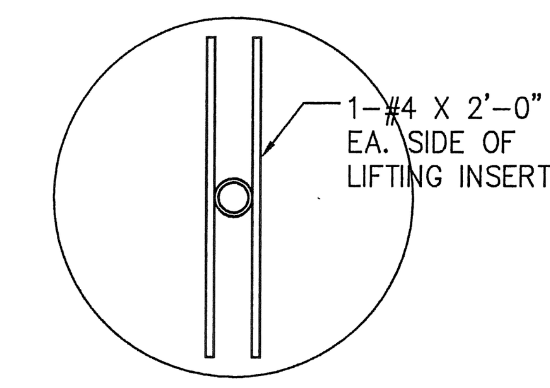
W1 W2,W3 W4

LIFT INSERTS NOTES

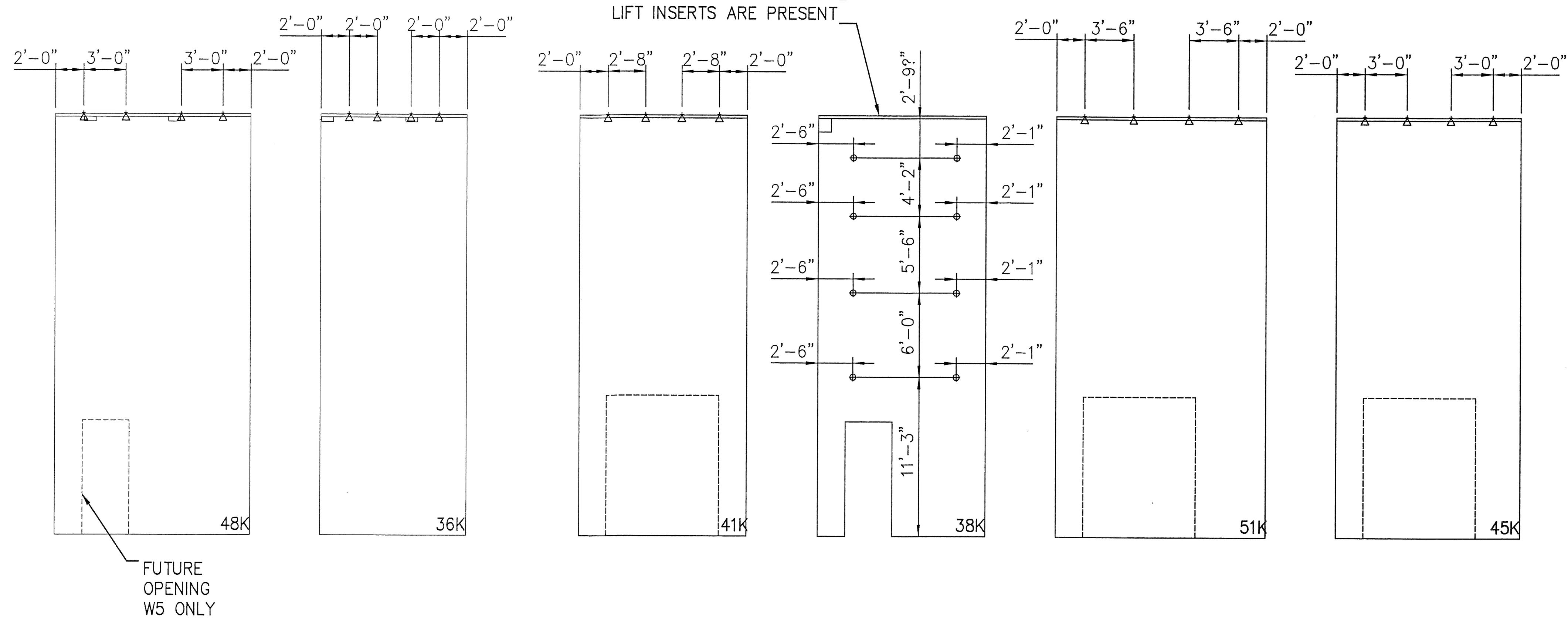
1. SYMBOLS:
   - INDICATES ?"¿ COIL INSERT
   - INDICATES PANEL LIFTING INSERT.
   - INDICATES PANEL EDGE LIFT INSERT.
2. MAXIMUM APPLIED STATIC WORKING LOADS TO LIFT INSERTS:

INSERTS	SHEAR (LBS.)	TENSION (LBS.)
5½"	7,500	5,500

   LIFTING INSERT CAPACITIES MUST MEET OR EXCEED THESE LOADS AND MUST HAVE A FACTOR OF SAFETY SUFFICIENT TO ALLOW FOR NORMAL DYNAMIC LOADS IMPOSED DURING LIFT.
3. LIFTING INSERT HEIGHT SHALL EQUAL STRUCTURAL PANEL THICKNESS, UNLESS NOTED. NO LIFTING INSERT SHALL BE PLACED CLOSER THAN 15" TO ANY PANEL EDGE, BLOCKOUT OR OPENING.
4. WALL PANELS SHALL NOT BE LIFTED UNTIL THE CONCRETE HAS ATTAINED A COMPRESSIVE STRENGTH OF 2,500 PSI AND A MINIMUM FLEXURAL STRENGTH OF 500 PSI AS DETERMINED BY STANDARD BEAMS FIELD CURED AND BROKEN IN ACCORDANCE WITH ASTM C78.
5. PANEL WEIGHTS SHOWN ARE FOR PANEL ONLY. NO ALLOWANCE HAS BEEN MADE FOR RIGGING WEIGHT.



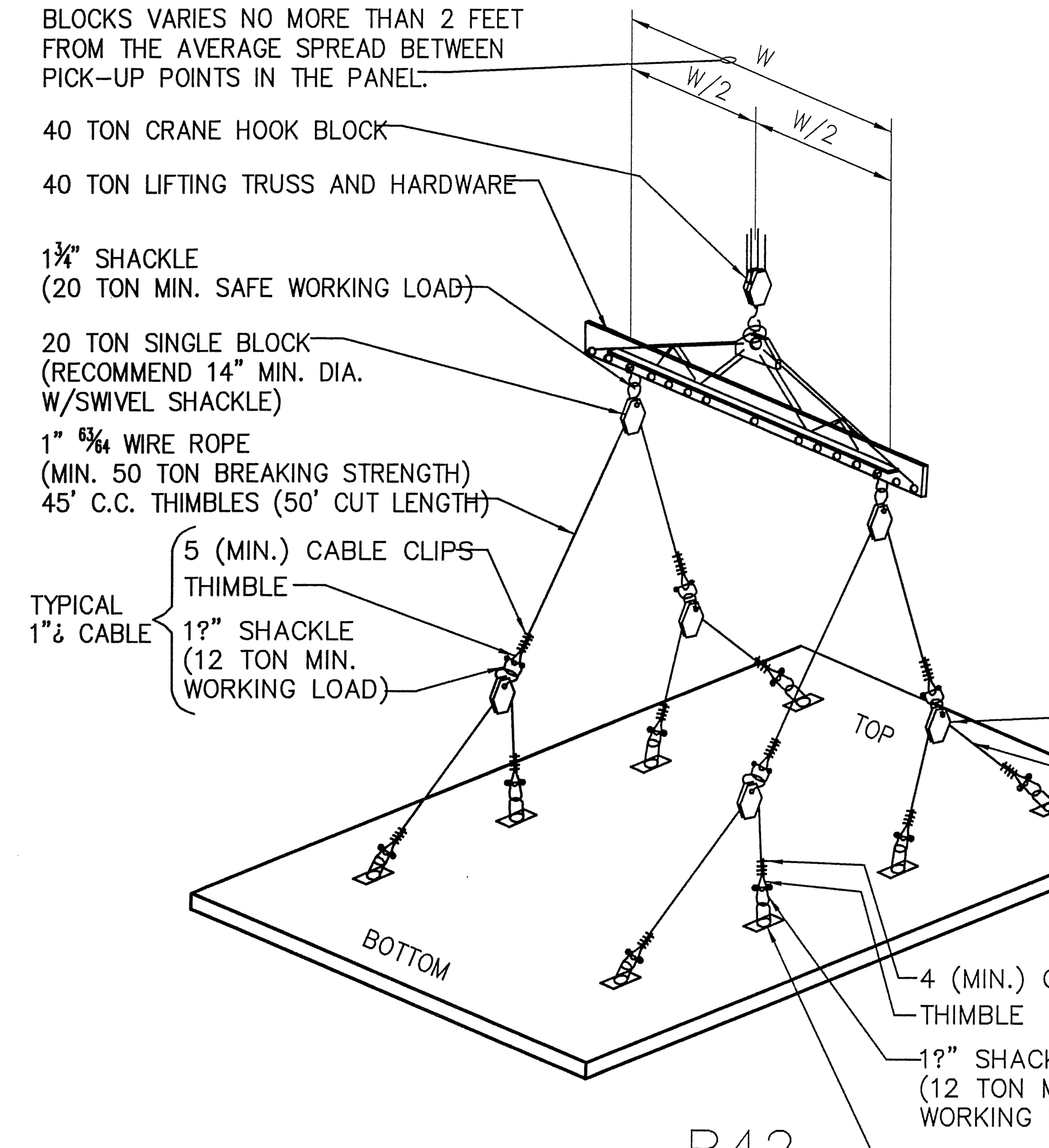
TYPICAL REINFORCING AT LIFTING INSERT



W5 THRU W8 W9 S13,S14,S16 S17,S18,S21 S15 S19,S20 S22

NOTE: PANELS W1 THRU W9 AND S13 THRU S22 USE EXISTING INSERTS AS SHOWN TO MOVE PANELS

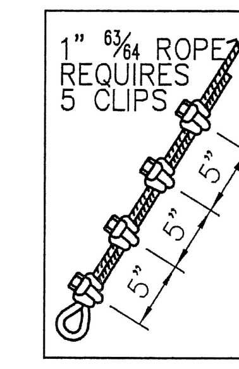
NOTE: LIFTING INSERT LOCATIONS FOR PANELS W1 THRU W9 AND S13 THRU S22 ARE LOCATED ASSUMING FUTURE OPENINGS HAVE NOT BEEN CUT IN PANELS



R42

NOTES:

1) INSPECT ALL RIGGING REGULARLY FOR SIGNS OF WEAR OR BREAKAGE.
2) CHECK AND TIGHTEN CABLE CLIPS REGULARLY AS ROPES UNDER HEAVY LOADS ARE GRADUALLY REDUCED IN DIAMETER.
3) LENGTHS AND SIZES SHOWN WILL ACCOMMODATE WALL PANELS TO 40 TONS IN WEIGHT WITH PICK-UP POINTS IN NORMAL CONFIGURATION.
4) RECOMMEND WIRE ROPE TO BE 6X25 CLASSIFICATION IMPROVED PLOW STEEL WITH WIRE ROPE CORE.
5) VERIFY LIFT INSERT CAPACITIES.
6) ALL SIZES SHOWN ARE TYPICAL FOR LEFT AND RIGHT SETS OF RIGGING. (SYMMETRICAL ABOUT CRANE HOOK BLOCK)



BASE OF CLIP MUST BEAR AGAINST LIVE END OF ROPE WITH U-BOLT FASTENED AROUND SHORT END.

BOLTS SHOULD BE RE-TIGHTENED UNDER LOAD.

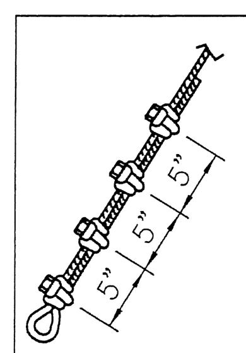
THIMBLE AND CLIPS MUST BE PROPERLY SIZED FOR ¾" ⁶³⁄₆₄ AND 1"⁶³⁄₆₄ ROPE.

ROPE END DETAIL

WALL PANEL CONCRETE VOLUME SCHEDULE

WALL PANEL(S)	NO. PANELS	FASCIA 1 PNL	FASCIA TOTAL	STRUCTURAL 1 PNL	STRUCTURAL TOTAL	TOTAL
ON8	1	1.02	1.02	2.19	2.19	3.21
ON9	1	1.86	1.86	4.03	4.03	5.88
OW4	1	2.09	2.09	4.42	4.42	6.51
OW5	1	1.57	1.57	3.28	3.28	4.85
OW6	1	2.19	2.19	4.59	4.59	6.78
TOTAL VOLUME						27.23

NOTE: VOLUME IS IN CUBIC YARDS



BASE OF CLIP MUST BEAR AGAINST LIVE END OF ROPE WITH U-BOLT FASTENED AROUND SHORT END.

BOLTS SHOULD BE RE-TIGHTENED UNDER LOAD.

THIMBLE AND CLIPS MUST BE PROPERLY SIZED FOR 1" ⁶³⁄₆₄ ROPE.

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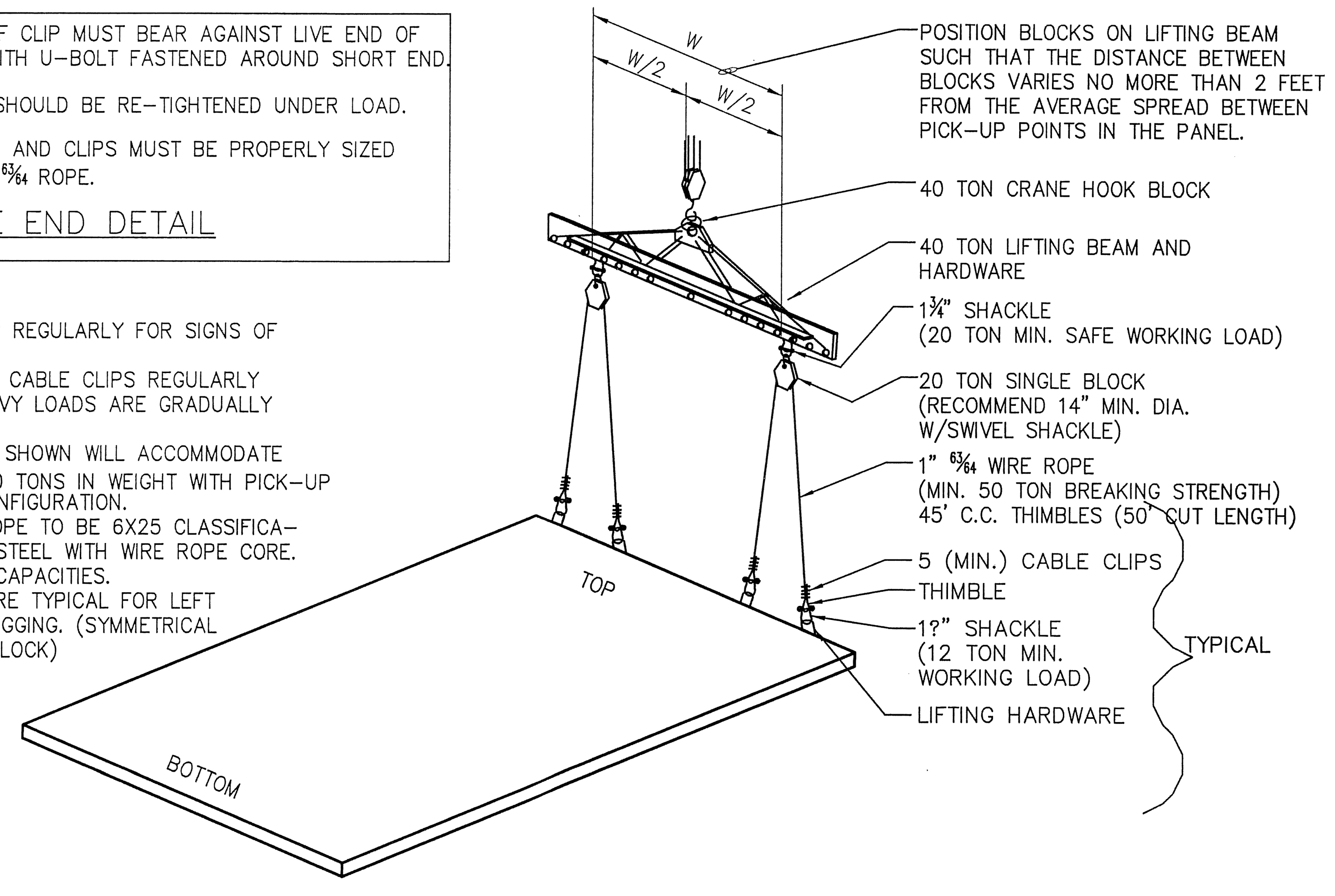
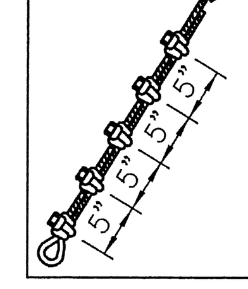


RE4H



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BOLTS SHOULD BE RE-TIGHTENED UNDER LOAD.

THIMBLE AND CLIPS MUST BE PROPERLY SIZED FOR 1" ⁶³⁄₆₄ ROPE.

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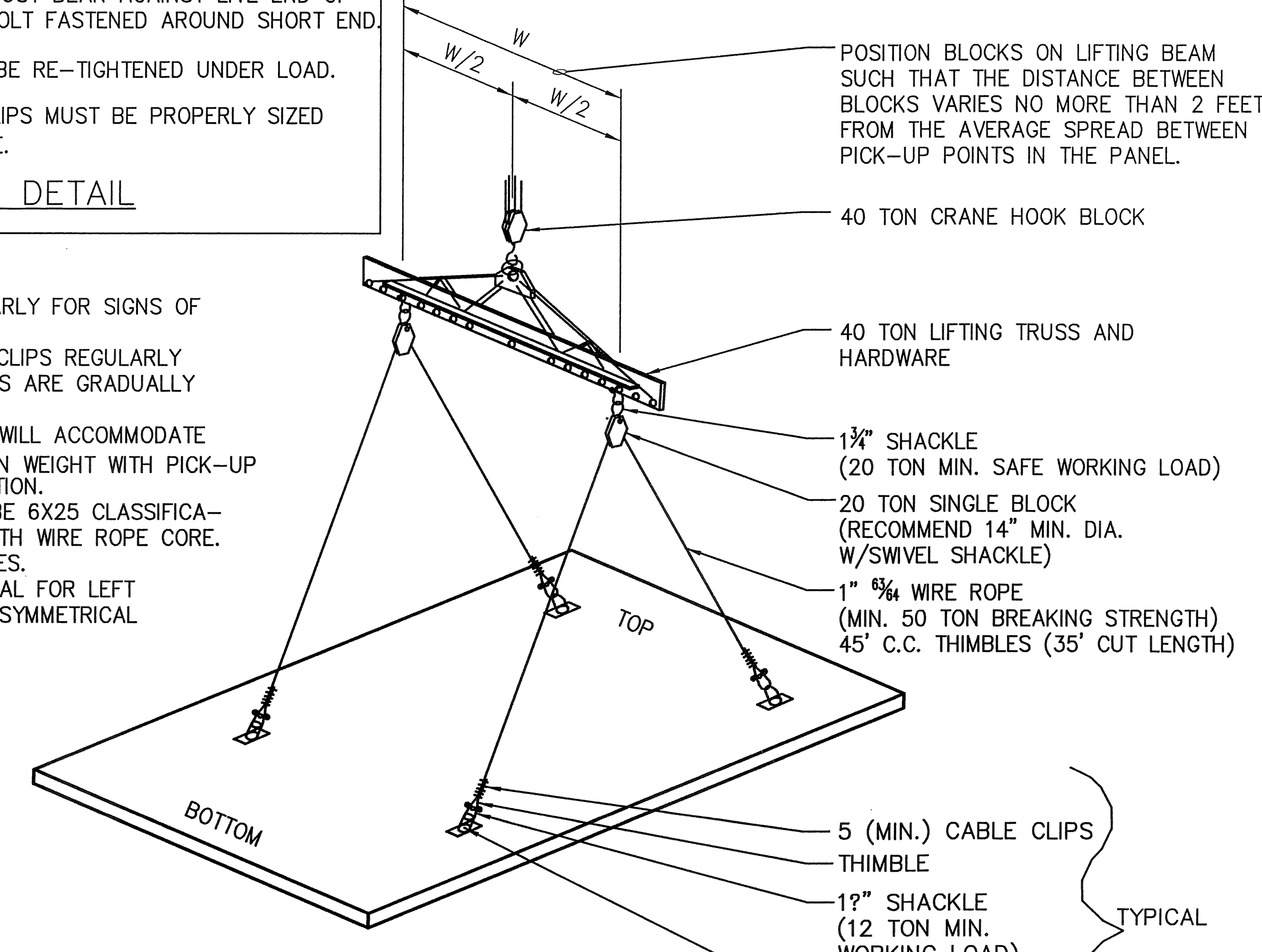

R22H

PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

NO.	DATE	DESCRIPTION	BY
2	2-10-09	RELEASED FOR CONSTRUCTION	STR
1	8-14-08	RELEASED FOR REVIEW	STR

REVISIONS

CON/STEEL® TILT-UP SYSTEMS (937) 256-9996

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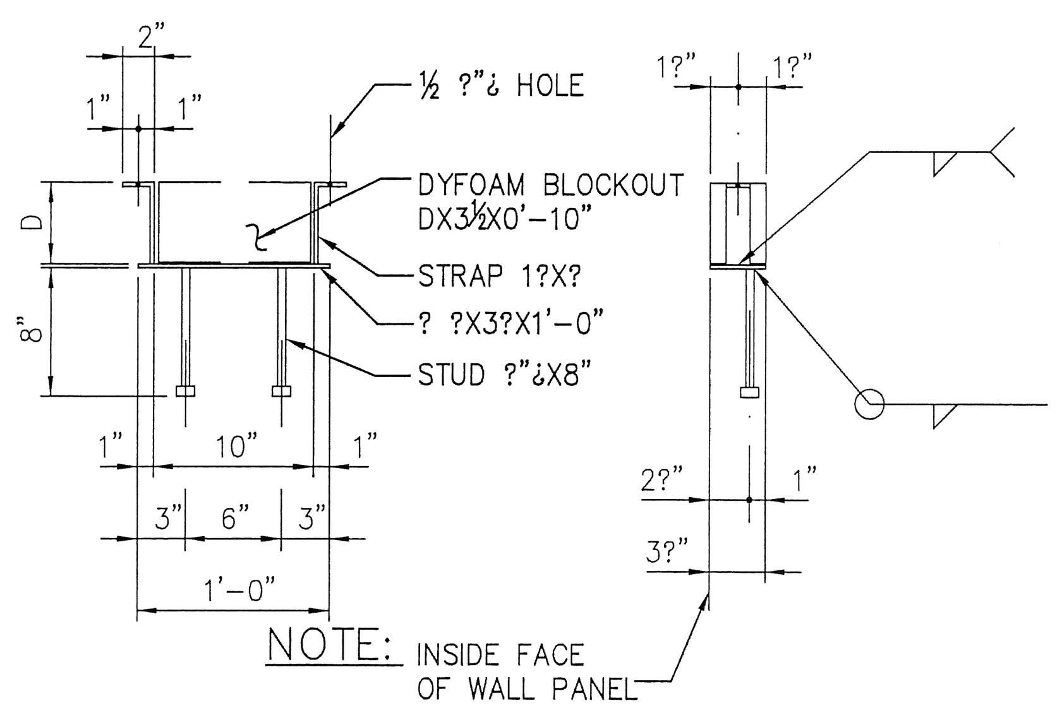
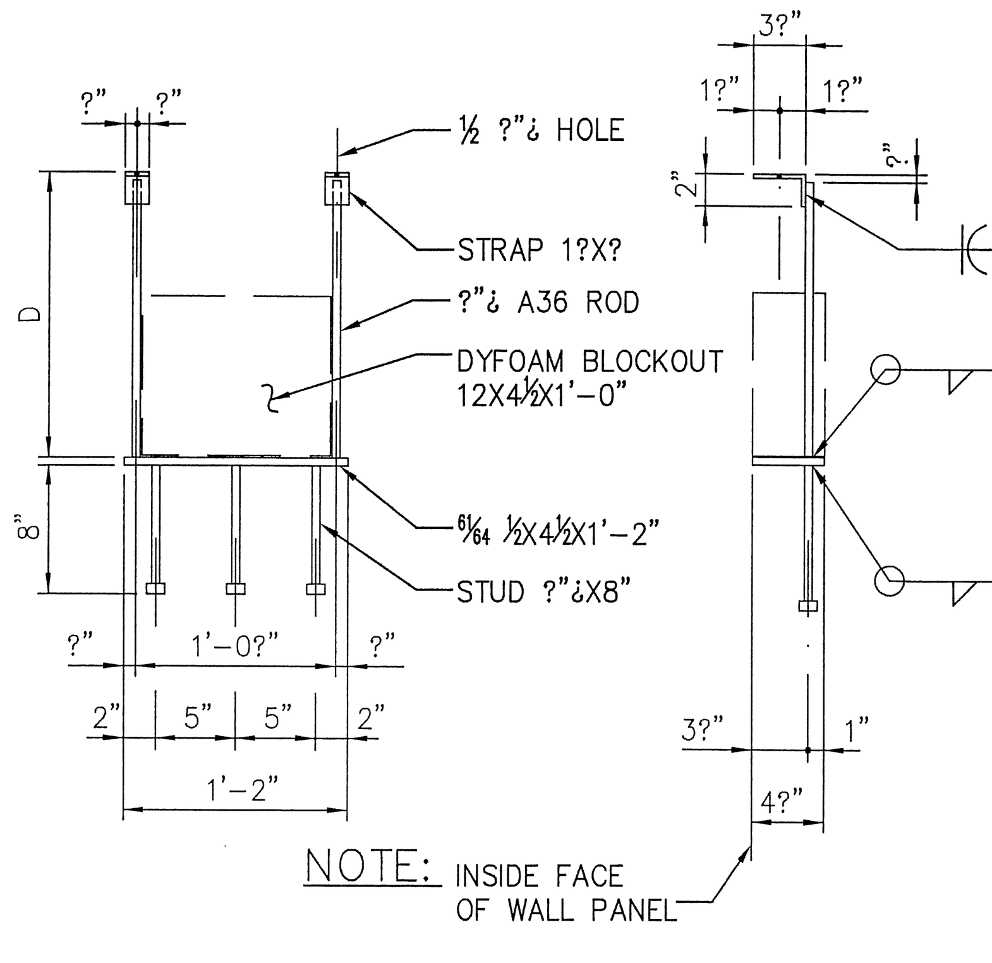
PACKAGE DISTRIBUTION CENTER ADDITIONS
3901 HANSON ROAD
MADISON, WISCONSIN

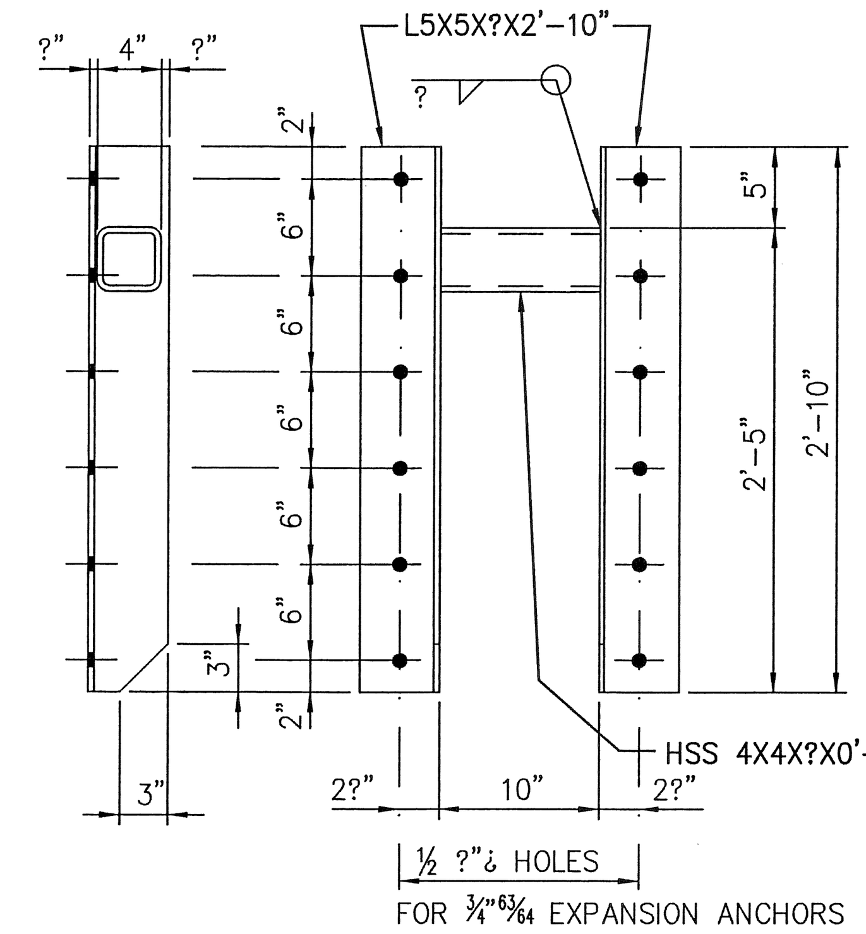
SHEET TITLE	WALL PANEL INSERTS
DESIGNED	JML
DRAWN	HMW
CHECKED	STR
DATE	AUG. 08
JOB NO.	103220
SHEET NO.	S600



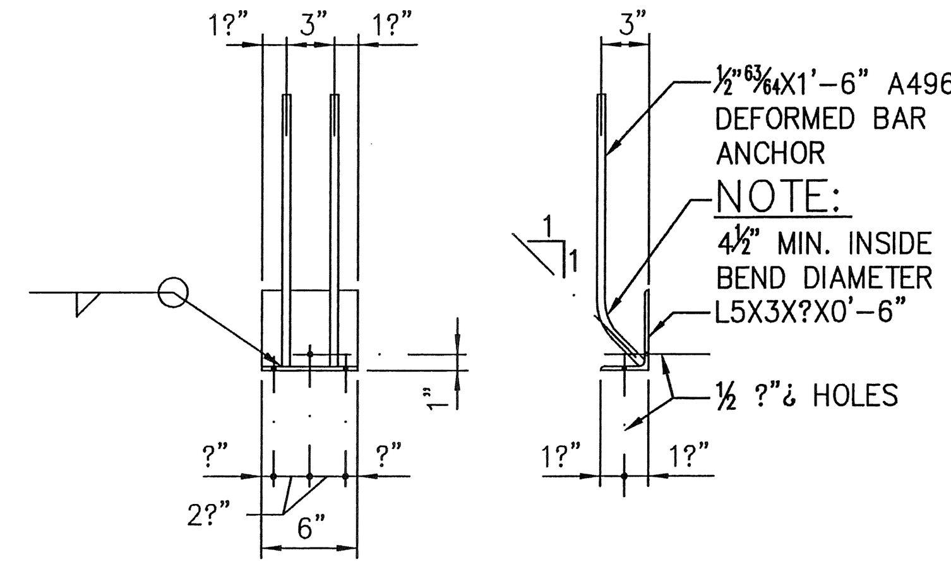
BAR JOIST SEAT
JS2
(QUANTITY 5)



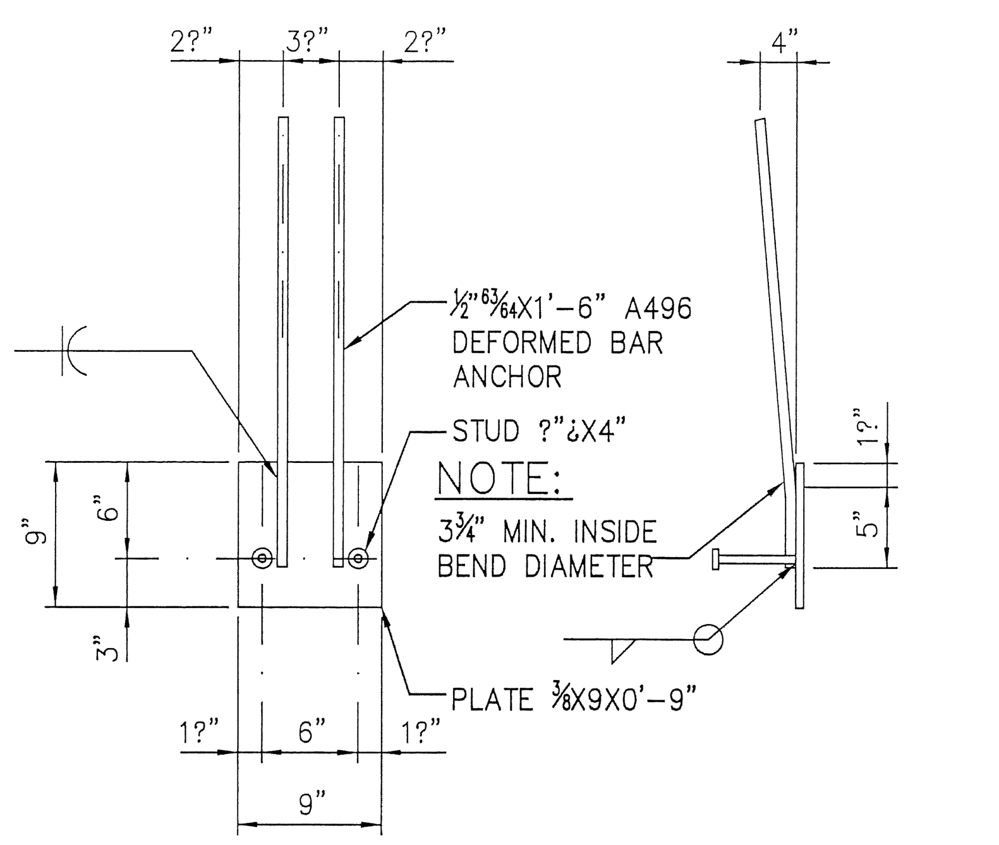
JOIST GIRDER/BEAM SEAT
GS2
(QUANTITY 2)



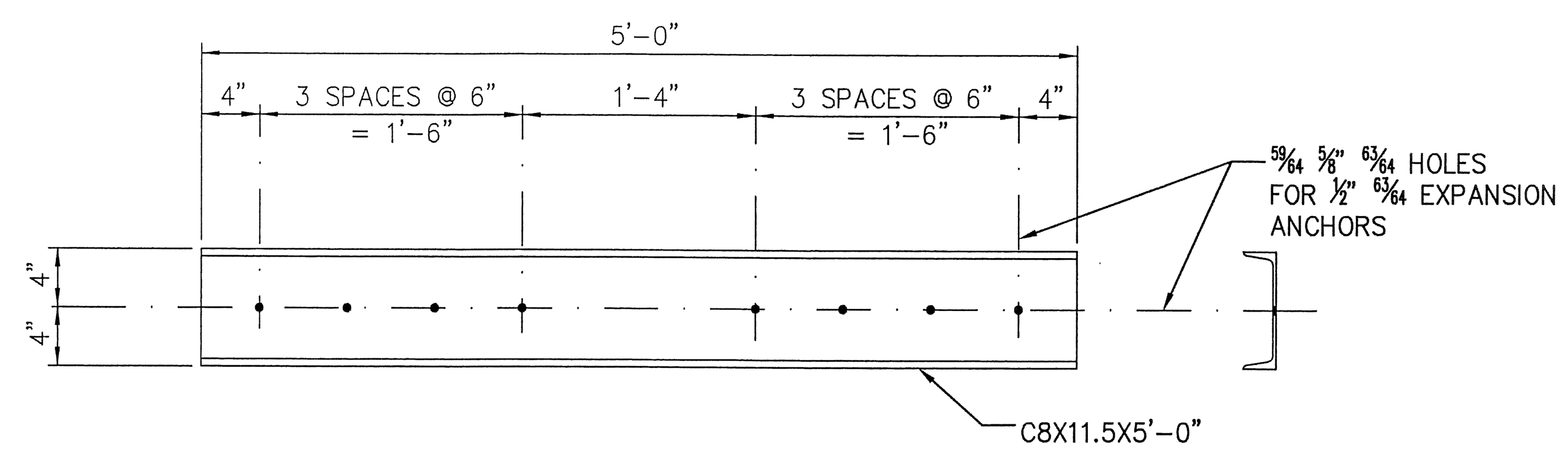
BEAM, JOIST GIRDER SEAT
GS5
(QUANTITY 1)



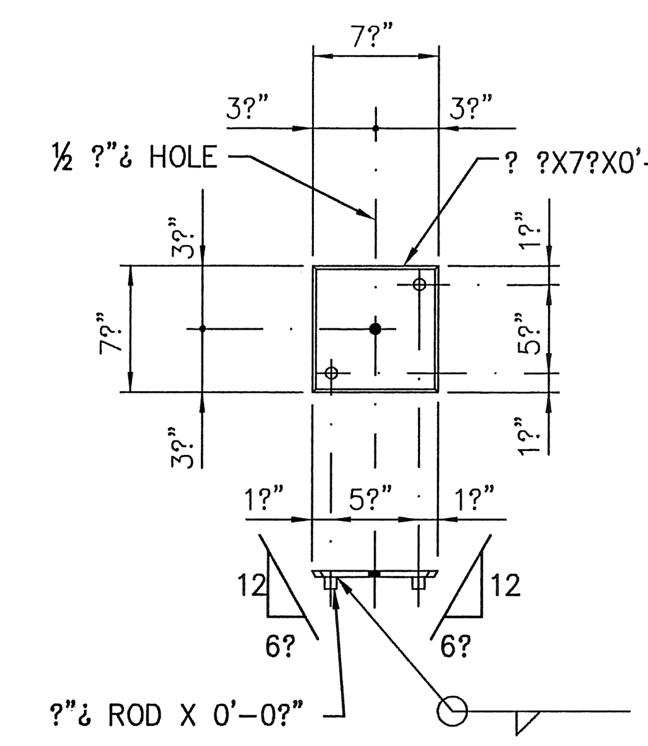
EMBEDDED ANGLE
EA1
(QUANTITY 13)



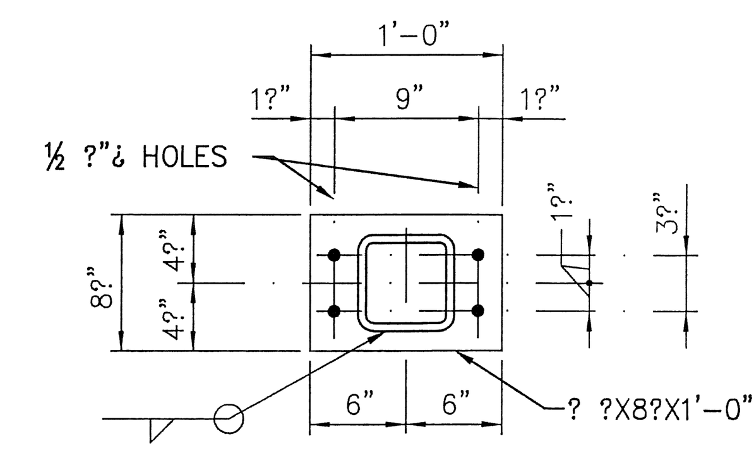
EMBEDDED PLATE
EP5
(QUANTITY 4)



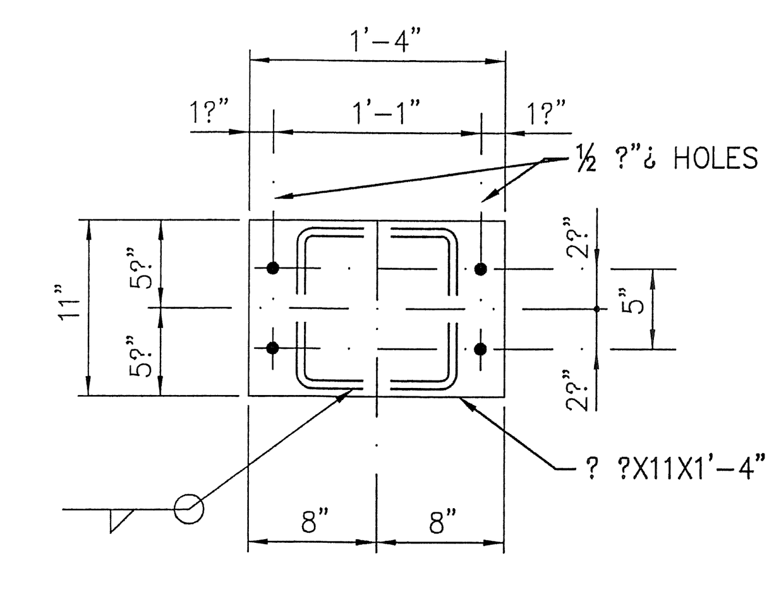
PANEL CONNECTOR
PC1
(QUANTITY 1)



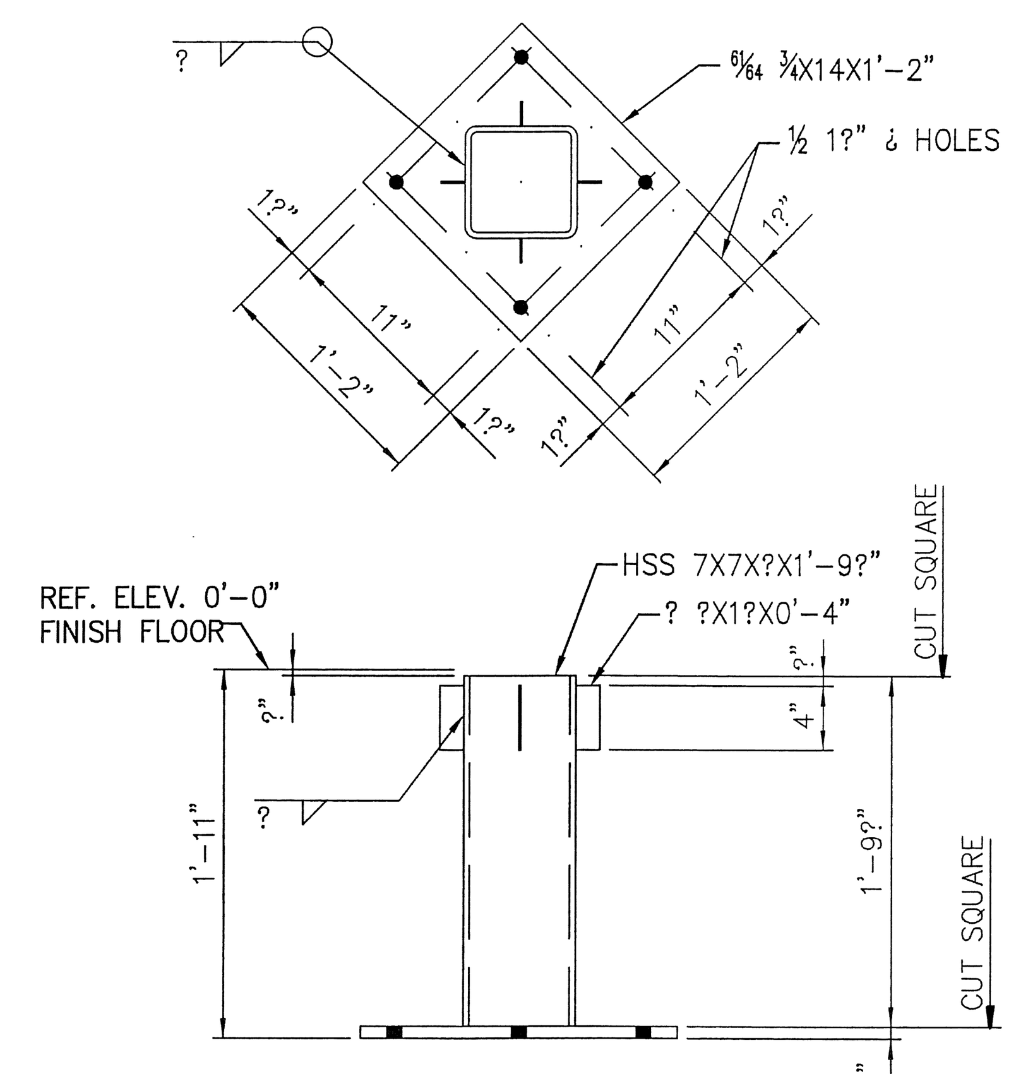
COLUMN BASE CAP
CC4
(QUANTITY 1)



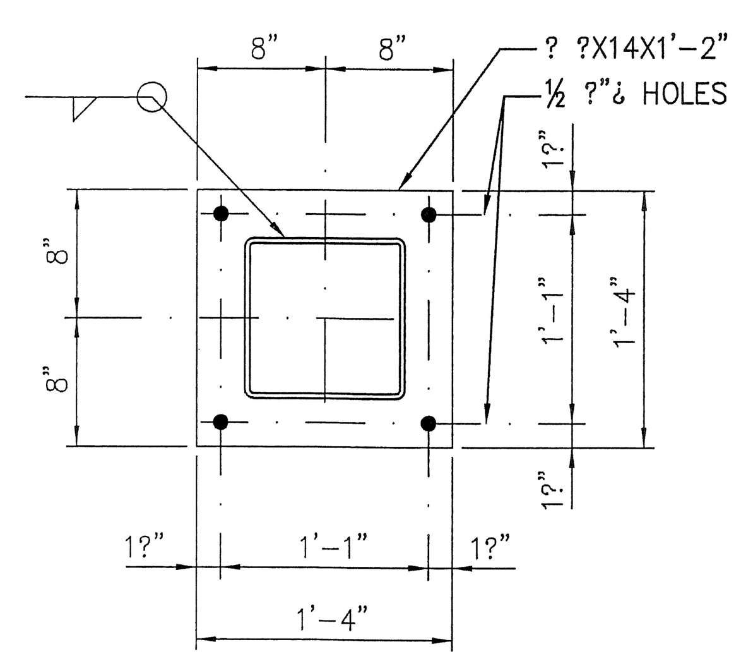
COLUMN CAP PLATE
C9A
(QUANTITY 1)



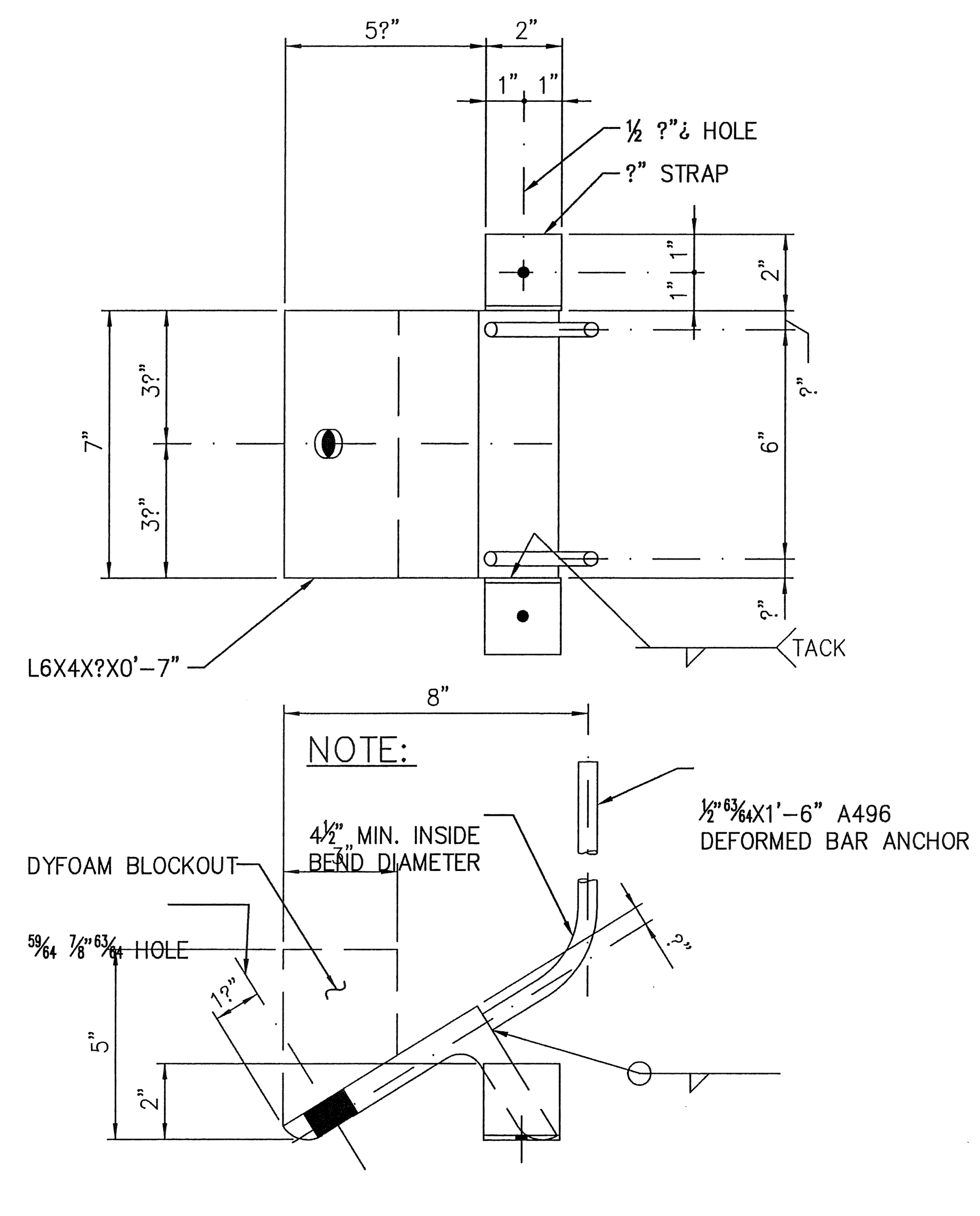
COLUMN CAP PLATE
C7D
(QUANTITY 4)



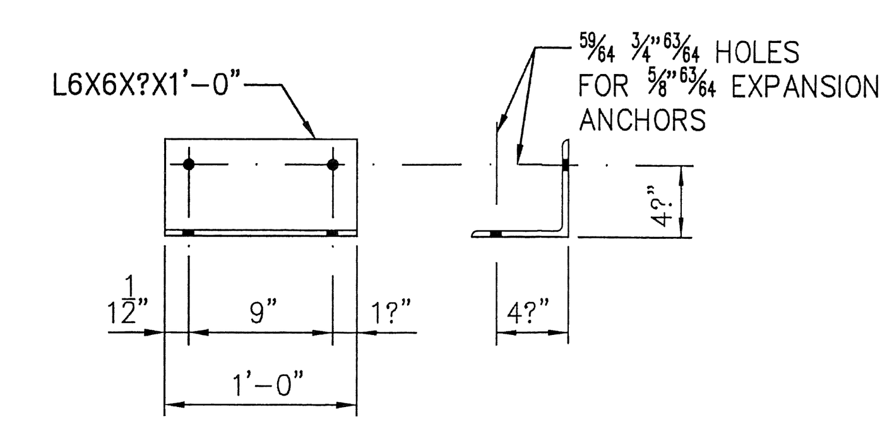
COLUMN BASE
CB4
(QUANTITY 1)



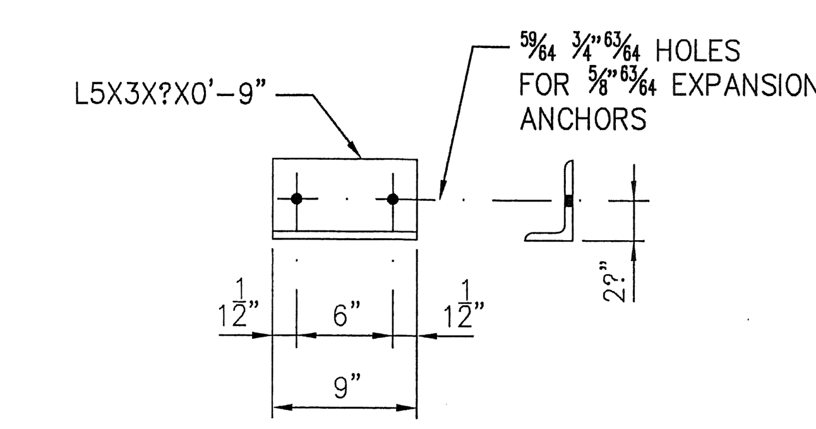
COLUMN BASE PLATE
B6
(QUANTITY 4)



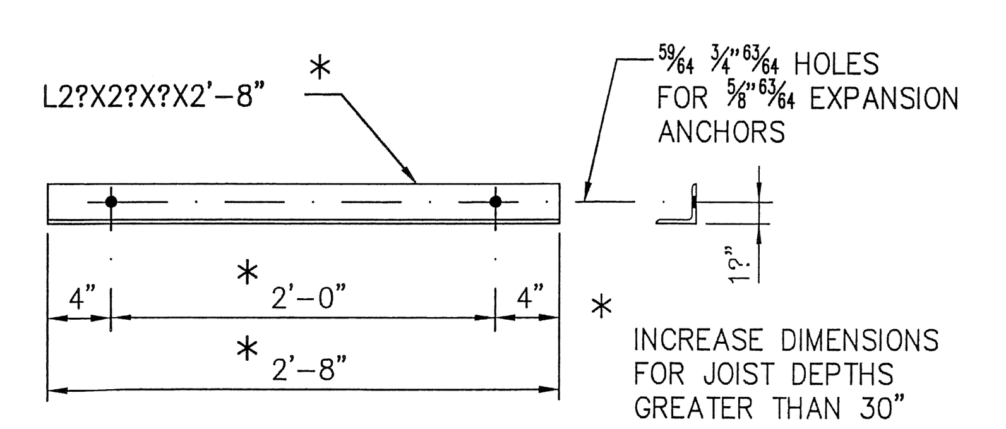
EMBEDDED BASE ANCHOR
U.S. PATENT NO. 5,609,005
EB4
(QUANTITY 10)



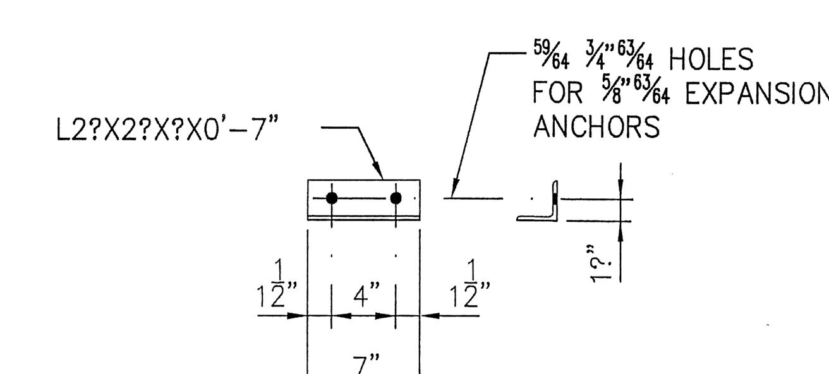
CORNER ANGLE
L1
(QUANTITY 5)



STABILIZER ANGLE
L3
(QUANTITY 2)



VERTICAL WALL BRACE ANGLE
L2
(QUANTITY 13)



BRIDGING BRACE ANGLE
L4
(QUANTITY 10)

PLAIN MATERIALS LIST

DESCRIPTION	QUANTITY	MARK
L4X4X?X0'-6"	4	L5

MISCELLANEOUS MATERIALS LIST

DESCRIPTION	QUANTITY	MARK
PERIMETER ANGLE L2?X2?X?	338 L.F.	
BOTTOM CHORD EXTENSION L2½X2½X¾ (FIELD MEASURE)	6 PCS.	
WALL BRACE L2?X2?X?X20'-0"	13 PCS.	
PERIMETER ANGLE L5X3X¼ W/⅜" X 3" SLOTTED HOLES @ 4'-0" O.C.	26 L.F.	
PERIMETER ANGLE L6X3?X?	52 L.F.	

NOTE: VERIFICATION OF QUANTITIES IS THE RESPONSIBILITY OF THE CONTRACTOR.

PROJECT RECORD 09.14.09

CON/STEEL ALLIANCE

NO.	DATE	DESCRIPTION	BY
1	2-10-09	RELEASED FOR CONSTRUCTION	STR

REVISIONS

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3901 HANSON ROAD
MADISON, WISCONSIN

SHEET TITLE: STANDARD FABRICATION DETAILS

DESIGNED	JML	JOB NO.	103220
DRAWN	JMH		
CHECKED	STR	SHEET NO.	S700
DATE	AUG. 08		